

**CITY OF DANA POINT
PLANNING COMMISSION
AGENDA REPORT**

DATE: MARCH 11, 2019

TO: DANA POINT PLANNING COMMISSION

FROM: COMMUNITY DEVELOPMENT DEPARTMENT
MATT SCHNEIDER, DIRECTOR OF COMMUNITY DEVELOPMENT
BELINDA DEINES, SENIOR PLANNER

SUBJECT: COASTAL DEVELOPMENT PERMIT CDP18-0013 TO CONSTRUCT A 146 SQUARE-FOOT ADDITION, INSTALLATION OF NEW WINDOWS, AND FOUNDATION REPAIR ON AN EXISTING 5,975 SQUARE-FOOT SINGLE-FAMILY DWELLING WITHIN THE RESIDENTIAL SINGLE FAMILY 4 (RSF 4) ZONING DISTRICT LOCATED AT 87 MONARCH BAY DRIVE

RECOMMENDATION: That the Planning Commission adopt the attached resolution approving Coastal Development Permit CDP18-0013.

APPLICANT: Tom and Barbara Stiles, Property Owner

REPRESENTATIVE: Morris Skenderian, Architect

REQUEST: Approval of a Coastal Development Permit to construct a 146 square-foot addition, new windows, and foundation repair within the City's Coastal Overlay District and the Appeals Jurisdiction of the California Coastal Commission.

LOCATION: 87 Monarch Bay Drive (APN 670-121-58, 670-121-59)

NOTICE: Notices of the Public Hearing were mailed to property owners within a 500-foot radius and occupants within a 100-foot radius on March 1, 2019 published within a newspaper of general circulation on March 1, 2019, and posted on March 1, 2019 at Dana Point City Hall, the Dana Point and Capistrano Beach Branch Post Offices, as well as the Dana Point Library.

ENVIRONMENTAL: Pursuant to the California Environmental Quality Act (CEQA), the project is found to be Categorically Exempt per Section 15301 (Class 1 – Existing Facilities) in that the project involves an addition less than 50 percent of the existing floor area.

ISSUES:

- Project consistency with the Dana Point General Plan, Dana Point Zoning Code (DPZC), and Local Coastal Program (LCP);
- Project compatibility with and enhancement of the site and surrounding neighborhood; and
- Project satisfaction of all findings required pursuant to the DPZC and LCP for approval of a Coastal Development Permit (CDP).

BACKGROUND:

The subject site is comprised of two lots with a combined lot area of 25,814 square feet. The site is located within Monarch Bay, which consists of an established and built-out neighborhood of single-family dwellings. The site is located on an oceanfront bluff and is bounded by other single-family dwellings, the Pacific Ocean to the south, and the City boundary to the west. Currently, the site is improved with an existing 5,975 square-foot, two-story single-family dwelling with an attached three-car garage. (Supporting Document 2). The property is located within the "Residential Single Family 4" (RSF 4) Zoning District, and is located within the Coastal Overlay District (the California Coastal Zone), and the Appeals Jurisdiction of the California Coastal Commission.

On December 3, 1997, the Planning Commission approved CDP97-20 to allow the remodel of the existing structure, new foundation, pool repair, and a balcony addition. The existing structure is legal, nonconforming because the structure encroaches into the 25' coastal bluff edge setback.

DISCUSSION:

The proposed scope of work involves a 146 square-foot addition within the existing building footprint, which involves conversion of lower level crawl space area to an elevator, hallway, and storage room. The existing powder room and closet area on the main level will be reconfigured to accommodate the new elevator adjacent to the entry way.

On the east corner of the lower level family room, the applicant proposes to install two new windows within the existing walls. This corner of the structure is legal, nonconforming and encroaches into the required 25' bluff edge setback. Pursuant to DPZC Section 9.63.030(b), improvements and maintenance to nonconforming structures that are conforming to use are encouraged to be maintained and aesthetically improved, including minor cosmetic improvements. The proposed windows will not expand the nonconforming structure. The applicant proposes to remove the interior non-bearing walls and remove a portion of the existing exterior bearing wall to create two openings for

the windows. The existing structural corner will remain and no new foundations are proposed within the bluff edge setback. The applicant has provided structural details to show the extent of the foundation repair below the windows. The new windows are proposed to match the existing material, color, and glazing of the other windows and sliding doors on the same elevations.

The proposed improvements are located within 50 feet of the coastal bluff edge. City staff has reviewed the geotechnical report for the proposed remodel and elevator additions. Additional development standards for this property are set forth in the City's Zoning Code Chapter 9.09 (Residential Development Standards) specifically for the RBR 12 zone. The project as designed complies with all applicable development standards, including setbacks, parking, and height limits. No deviations are requested. Table 1 summarizes applicable RSF 12 zoning designation development standards and the project's conformance with those requirements:

Table 1: Compliance with RSF 4 Development Standards

Development Standard	Requirement	Existing	Compliant with Standard
Front Setback	20 feet	8 feet	No
Side Setbacks	5 feet	5 feet	Yes
Bluff Edge Setback	25 feet	17 feet	No
Height	24 feet	22 feet	Yes
Parking Required	3 covered parking spaces minimum for five bedrooms	3 car garage	Yes

Coastal Development Permit CDP18-0013

Pursuant to Section 9.69.040 of the Dana Point Zoning Code, any improvements within 50 feet of the coastal bluff edge and located in the City's Coastal Overlay District Appeals Jurisdiction of the California Coastal Commission requires approval of a Coastal Development Permit (CDP). The proposed 146 square-foot lower level addition, new windows, and foundation repair are located within 50 feet of the coastal bluff edge and comply with the applicable development standards of the RSF 4 Zoning District. The project would not result in any landform alterations to the site.

Section 9.69.070 of the DPZC stipulates a minimum of seven (7) findings to approve a Coastal Development Permit, requiring that the project:

- 1. Be in conformity with the certified Local Coastal Program as defined in Chapter 9.75 of this Zoning Code. (Coastal Act/30333, 30604(b); 14 CA Code of Regulations/13096).*

2. *If located between the nearest public roadway and the sea or shoreline of any body of water, be in conformity with the public access and public recreation policies of Chapter Three of the Coastal Act. (Coastal Act/30333, 30604(c); 14 CA Code of Regulations/13096).*
3. *Conform with Public Resources Code Section 21000 and following, and there are no feasible mitigation measures or feasible alternatives available which would substantially lessen any significant adverse impact that the activity may have on the environment. (Coastal Act/30333; 14 CA Code of Regulations/13096).*
4. *Be sited and designed to prevent adverse impacts to environmentally sensitive habitats and scenic resources located in adjacent parks and recreation areas, and will provide adequate buffer areas to protect such resources.*
5. *Minimize the alterations of natural landforms and not result in undue risks from geologic and erosional forces and/or flood and fire hazards.*
6. *Be visually compatible with the character of surrounding areas, and, where feasible, will restore and enhance visual quality in visually degraded areas.*
7. *Conform to the General Plan, Zoning Code, applicable Specific Plan, Local Coastal Program, or any other applicable adopted plans and programs.*

The recommended findings for approval of the CDP are outlined in the draft Resolution No. 19-03-11-XX, attached to this report as Action Document 1.

CORRESPONDENCE: On October 29, 2018, the project received approval from the Monarch Bay Association Architectural Review Committee.

CONCLUSION: Staff finds that the proposed project is consistent with the policies and provisions of the City of Dana Point General Plan, Dana Point Zoning Code, and Local Coastal Program. As the project has been found to comply with all standards of development, staff recommends the Planning Commission adopt the attached draft Resolution, approving Coastal Development Permit 18-0013 subject to the findings and conditions of approval contained therein.


Belinda Deines, Senior Planner


Matt Schneider, Director
Community Development Department

ATTACHMENTS:

Action Documents

1. Draft Planning Commission Resolution No. 19-03-11-xx

Supporting Documents

2. Vicinity Map
3. Site Photos
4. Architectural Plans

Action Document 1: Draft Planning Commission Resolution No. 19-03-11-xx

RESOLUTION NO. 19-03-11-XX

A RESOLUTION OF THE PLANNING COMMISSION OF THE CITY OF DANA POINT, CALIFORNIA, APPROVING COASTAL DEVELOPMENT PERMIT CDP18-0013 TO CONSTRUCT A 146 SQUARE-FOOT ADDITION, INSTALLATION OF NEW WINDOWS, AND FOUNDATION REPAIR ON AN EXISTING 5,975 SQUARE-FOOT SINGLE-FAMILY DWELLING WITHIN THE RESIDENTIAL SINGLE FAMILY 4 (RSF 4) ZONING DISTRICT LOCATED AT 87 MONARCH BAY DRIVE

The Planning Commission for the City of Dana Point does hereby resolve as follows:

WHEREAS, Morris Skenderian, Architect, (the "Representative") has filed an application on behalf of Tom and Barbara Stiles, ("Applicant"), the owners of real property commonly referred to as 87 Monarch Bay Drive (APN 670-121-58, 670-121-59) (the "Property"); and

WHEREAS, the Representative filed a verified application for a Coastal Development Permit to allow construction of a 146 square-foot addition, installation of new windows, and foundation repair; and

WHEREAS, said verified application constitutes a request as provided by Title 9 of the Dana Point Municipal Code; and

WHEREAS, pursuant to the California Environmental Quality Act (CEQA), the project is Categorically Exempt per Section 15301 (Class 1 – Existing Facilities) in that the application involves an addition less than 50 percent of the existing floor area; and

WHEREAS, the Planning Commission did, on the 11th day of March, 2019, hold a duly noticed public hearing as prescribed by law to consider said request; and

WHEREAS, at said public hearing, upon hearing and considering all testimony and arguments, if any, of all persons desiring to be heard, said Commission considered all factors relating to Coastal Development Permit CDP18-0013.

NOW, THEREFORE, BE IT HEREBY RESOLVED by the Planning Commission of the City of Dana Point as follows:

- A. That the above recitations are true and correct and incorporated herein by this reference.
- B. Based on the evidence presented at the public hearing, the Planning Commission adopts the following findings and approves CDP18-0013 subject to the following conditions of approval:

Findings:

Coastal Development Permit CDP18-0013

PLANNING COMMISSION RESOLUTION NO. 19-03-11-XX
CDP18-0013
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1. That the project is in conformity with the certified Local Coastal Program as defined in Chapter 9.75 of this Zoning Code. (Coastal Act/30333, 30604(b); 14 Cal. Code of Regulations/13096) **in that the proposed improvements are found to comply with all development standards of the Dana Point General Plan and Zoning Code (the latter acting as the Local Coastal Program Implementation Plan for the property) and will not result in any significant exterior modifications to the existing structure.**
2. If located between the nearest public roadway and the sea or shoreline of any body of water, that the project is in conformity with the public access and public recreation policies of Chapter Three of the Coastal Act. (Coastal Act/30333, 30604(c); 14 Cal. Code of Regulations/13096) **in that while the project is located between the nearest public roadway and the sea or shoreline, the property is an already developed lot zoned for residential use, located within a private community that does not contain public access ways or areas of recreation. Moreover, adequate public access to public tidelands or areas of recreation exist nearby at City, County and State beaches and accordingly, the project conforms to the public access and recreation policies of Chapter Three of the California Coastal Act.**
3. That the project conforms to Public Resources Code Section 21000 (the California Environmental Quality Act - CEQA) and following, that there are no feasible mitigation measures or feasible alternatives available which would substantially lessen any potentially significant adverse impact that the activity may have on the environment. (Coastal Act/30333; 14 Cal. Code of Regulations/13096) **in that the project qualifies as Categorically Exempt from review under CEQA pursuant to Section 15301 (Class 1 – Existing Facilities) in that the application involves an addition less than 50 percent of the existing floor area.**
4. That the project has been sited and designed to prevent adverse impacts to environmentally sensitive habitats and scenic resources located in adjacent parks and recreation areas, and will provide adequate buffer areas to protect such resources **in that the subject property lies adjacent to the Pacific Ocean, a noted sensitive habitat area, park and recreation area. However, the proposed addition is sited below grade and within the building footprint of the existing structure. The new windows and foundation repair are proposed on existing walls that were established as legal, nonconforming to the 25' coastal bluff setback. Proposed improvements are permitted by the Dana Point Zoning Code and Local Coastal Program. The project has been reviewed and found by City staff to conform to all development and geotechnical standards. Therefore, no buffer areas (from adjacent ESHA) are required.**

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5. That the project minimizes the alteration of natural landforms and will not result in undue risks from geologic and erosional forces and/or flood and fire hazards in that the proposed site improvements have been reviewed for geotechnical feasibility, including slope stability, edge retreat, and seismic considerations. City staff has reviewed and approved the geotechnical report dated January 18, 2019 (on file with the City) as certified by a registered engineering geologist and a geotechnical engineer. The report concludes that the proposed site improvements are feasible and provides design, construction, and maintenance recommendations to minimize risks from any potential geologic and/or erosional or flood forces.
6. That the project is visually compatible with the character of surrounding areas, and, where feasible, will restore and enhance visual quality in visually degraded areas in that the proposed site improvements are within an established community of identical uses. The proposed improvements conform to all standards of development prescribed by its respective zoning district. This conforming addition and repair to the project site constitutes fulfillment of General Plan Land Use and Zoning Code intent for the site, and accordingly enhancement of the property.
7. That the project conforms with the General Plan, Zoning Code, applicable Specific Plan, Local Coastal Program, or any other applicable adopted plans and programs in that the subject project has been reviewed by Planning and Building/Safety Division staff as well as the Public Works/Engineering Department and found to conform with applicable requirements of the Dana Point Zoning Code (which serves as the implementing document for the General Plan and Local Coastal Program Implementation Plan for the subject property). There are no adopted specific plans that apply to the subject property.

Conditions:

General:

1. Approval of this application permits construction of a 146 square-foot living area addition, installation of new windows, and foundation repair at 87 Monarch Bay Drive in accordance with the plans on file with the Community Development Department. Subsequent submittals for this project shall be in substantial compliance with the plans presented to the Planning Commission, and in compliance with the applicable provisions of the Dana Point General Plan, Local Coastal Program Implementation Plan and Zoning Code.

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2. This resolution shall be copied in its entirety, placed directly onto a separate plan sheet behind the cover sheet of any plans submitted to the City of Dana Point Building/Safety Division for plan check.
3. Approval of this application is valid for a period of 24 months (two years) from the noted date of determination. If the development approved by this action is not established, or a building permit for the project is not issued within such period of time, the approval shall expire and shall thereafter be null and void.
4. The application is approved as a plan for the location and design of the uses, structures, features, and materials shown on the approved plans. Any demolition beyond that described in the approved plans or any relocation, alteration, or addition to any use, structure, feature, or material, not specifically approved by this application, will nullify this approving action. If any changes are proposed regarding the location of, or alteration to the appearance or use of any structure, an amendment to this permit shall be submitted for approval by the Director of Community Development. If the Director determines that the proposed change complies with the provisions, spirit and intent of this approval action, and that the action would have been the same for the amendment as for the approved site plan, he/she may approve the amendment without requiring a new public hearing.
5. Failure to abide by and faithfully comply with any and all conditions attached to the granting of this permit shall constitute grounds for revocation of said permit.
6. The Applicant or any successor-in-interest shall defend, indemnify, and hold harmless the City of Dana Point ("CITY"), its agents, officers, or employees from any claim, action, or proceeding against the CITY, its agents, officers, or employees to attack, set aside, void, or annul an approval or any other action of the CITY, its advisory agencies, appeal boards, or legislative body concerning the project. Applicant's duty to defend, indemnify, and hold harmless the City shall include paying the CITY's attorney fees, costs and expenses incurred concerning the claim, action, or proceeding.

The Applicant or any successor-in-interest shall further protect, defend, indemnify and hold harmless the City, its officers, employees, and agents from any and all claims, actions, or proceedings against the City, its officers, employees, or agents arising out of or resulting from the negligence of the Applicant or the Applicant's agents, employees, or contractors. Applicant's duty to defend, indemnify, and hold harmless the City shall include paying the CITY's attorney fees, costs and expenses incurred concerning the claim, action, or proceeding. The Applicant shall also reimburse the City for City Attorney fees and costs associated with

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the review of the proposed project and any other related documentation.

The Applicant, and their successors-in-interest, shall be fully responsible for knowing and complying with all conditions of approval, including making known the conditions to City staff for future governmental permits or actions on the project site.

7. The project shall meet all water quality requirements including Low Impact Development (LID) implementation.
8. The Applicant, or Applicant's agent(s), shall be responsible for coordination with water district, sewer district, SDG&E, AT&T California and Cox Communication Services for the provision of water, sewer, electric, cable television and telephone and services. The Applicant, or Applicant's agent(s), shall be responsible for coordinating any potential conflicts or existing easements.
9. The Applicant shall exercise special care during the construction phase of this project. The applicant shall provide erosion and sediment control. The erosion control measures shall be constructed prior to the start of any other grading operations. The applicant shall maintain the erosion and sediment control devices until the final approval for all permits.
10. The Applicant, Applicant's agent(s), or successor-in-interest, shall prepare a Waste Management Plan to the City's C&D official per the Dana Point Municipal Code. A deposit will be required upon approval of the Waste Management Plan to ensure compliance. The Waste Management Plan shall indicate the estimated quantities of material to be recycled and the locations where the material is to be taken for recycling.
11. The Applicant shall limit all construction activities within the coastal bluff top setback area. The coastal bluff shall be protected at all times from potential erosion and construction activity.
12. The bluff edge setback shall be clearly shown on all plans submitted for review and approval.
13. Per Dana Point Municipal Code Section 9.27.030, no new structure foundations or improvements requiring a building permit will be allowed within the bluff edge setback. Review of the submitted plans indicates that all work associated with the proposed building addition and remodel will be supported by the existing foundation system of the residence, which is shown behind the indicated bluff edge setback line.
14. Please note that no new permitted foundations will be allowed within the bluff edge setback. All design professionals should evaluate the existing foundation system for the existing structure as it relates to the proposed

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windows at the seaward side of the residence in order to verify that new foundation elements will not be required as part of the proposed construction.

Prior to Issuance of a Building Permit:

15. Building plan check submittal shall include two (2) sets of the following construction documents: building plans (4 sets), energy calculations, structural calculations, and soils/geology report.
16. All documents prepared by a professional shall be wet-stamped and signed.
17. The Applicant, or Applicant's agent(s), shall cause the preparation and submittal of a geotechnical report in compliance with City of Dana Point standards for review and approval.
18. All submitted plans shall reflect the determined coastal bluff edge and all associated setbacks, as shown on the approved geotechnical report dated January 18, 2019 as a part of the Coastal Development Permit.

Prior to Final Approval of All Permits:

19. All landscaping and/or structural best management practices (BMPs) shall be constructed and installed in conformance with approved plans and specifications.
20. The final condition of the bluff edge setback shall be in accordance with Dana Point Municipal Code Section 9.27.030, with no new structure foundations or improvements requiring a building permit within the bluff edge setback.
21. The Applicant, or Applicant's agent(s), shall cause the scheduling of a final onsite inspection with the Community Development Department that shall include a review of compliance with any outstanding project conditions of approval.

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PASSED, APPROVED, AND ADOPTED at a regular meeting of the Planning Commission of the City of Dana Point, California, held on this 11th day of March, 2019 by the following vote, to wit:

AYES:

NOES:

ABSENT:

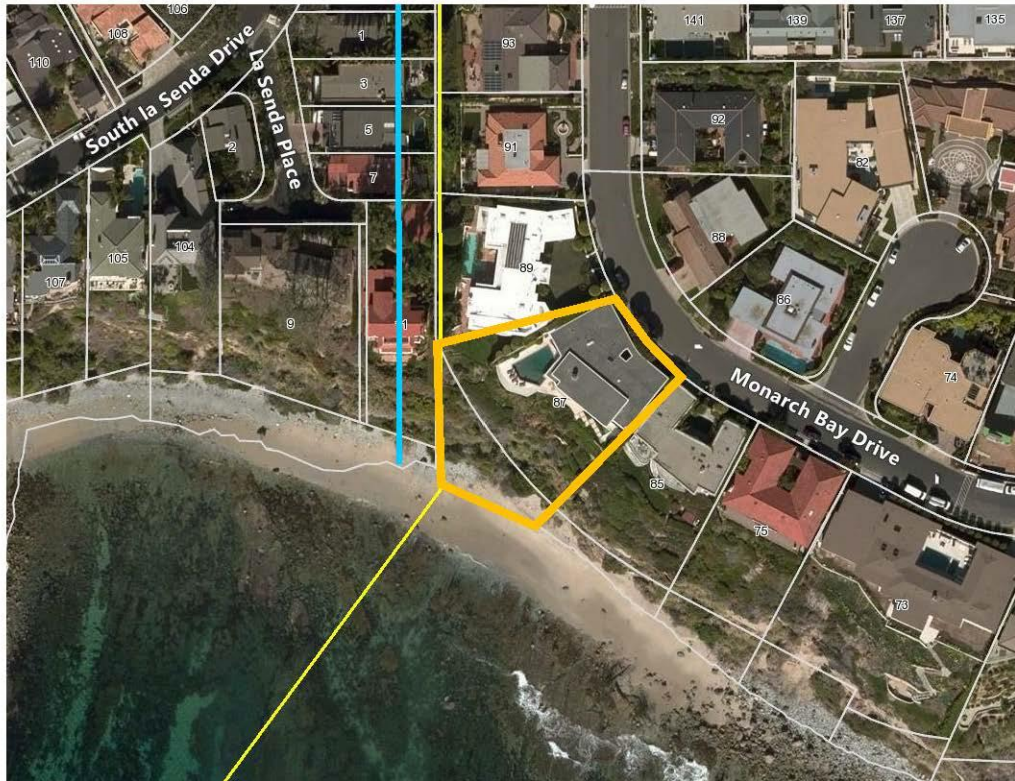
ABSTAIN:

Roy Dohner, Chairperson
Planning Commission

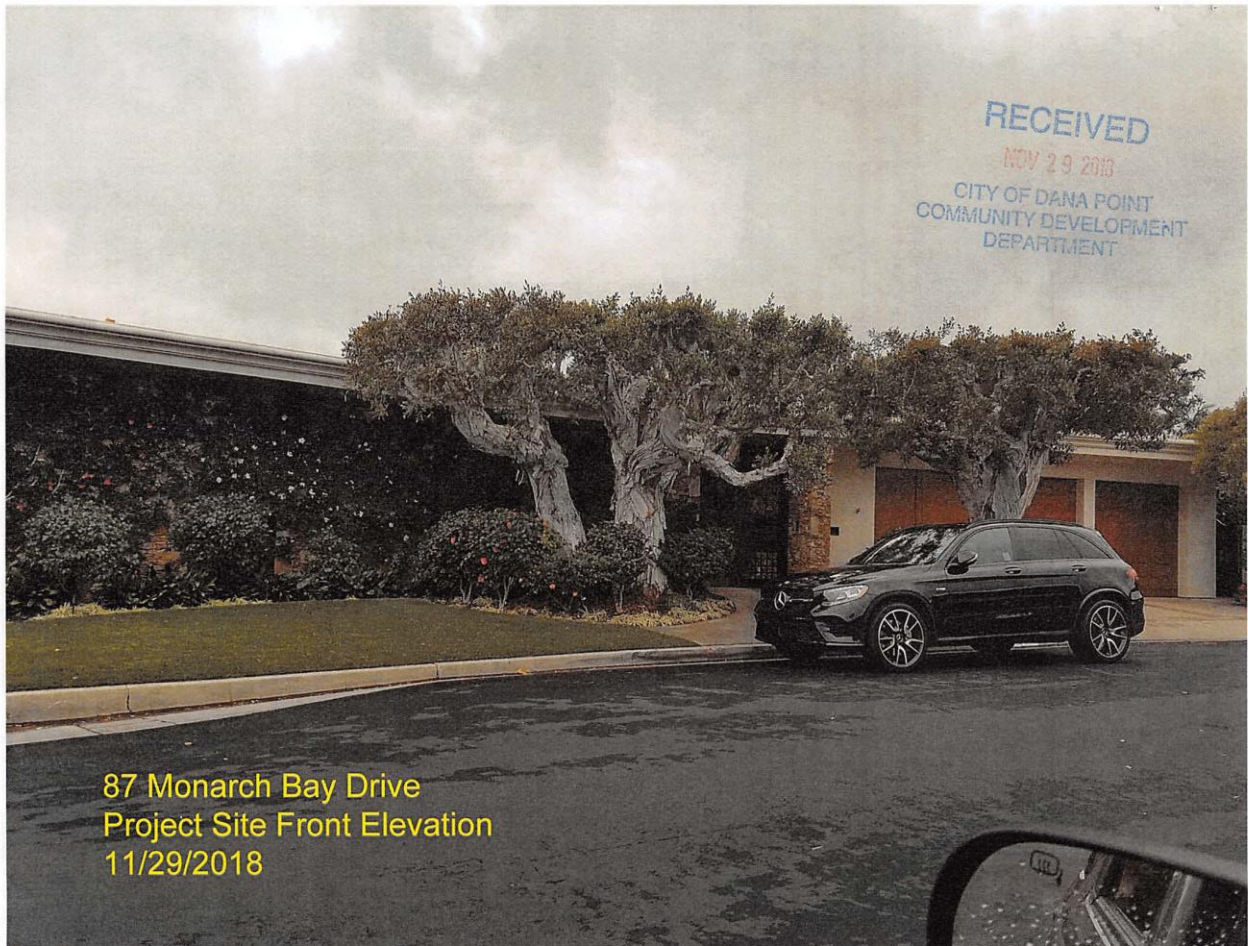
ATTEST:

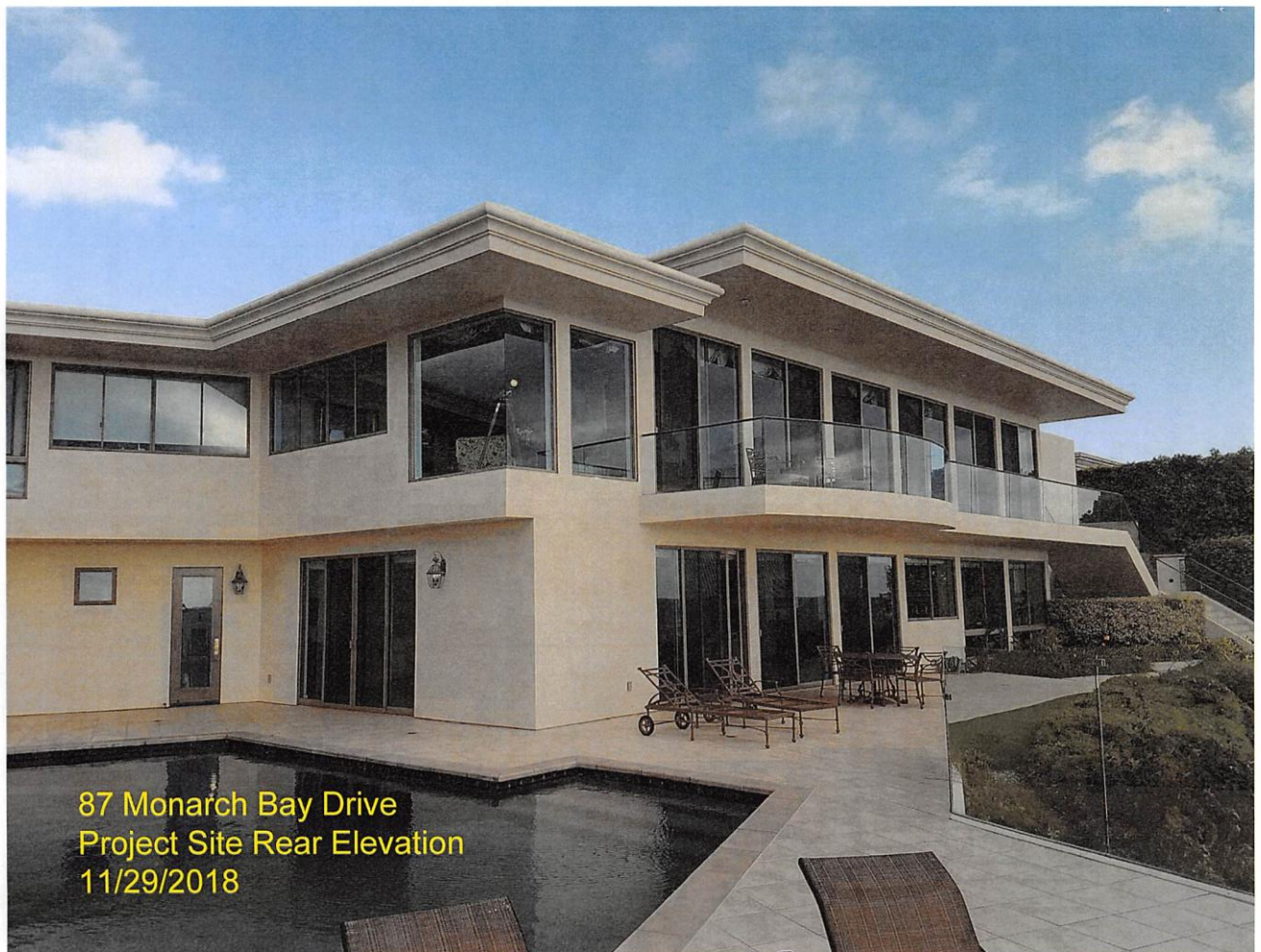
Matthew Schneider, Director
Community Development Department

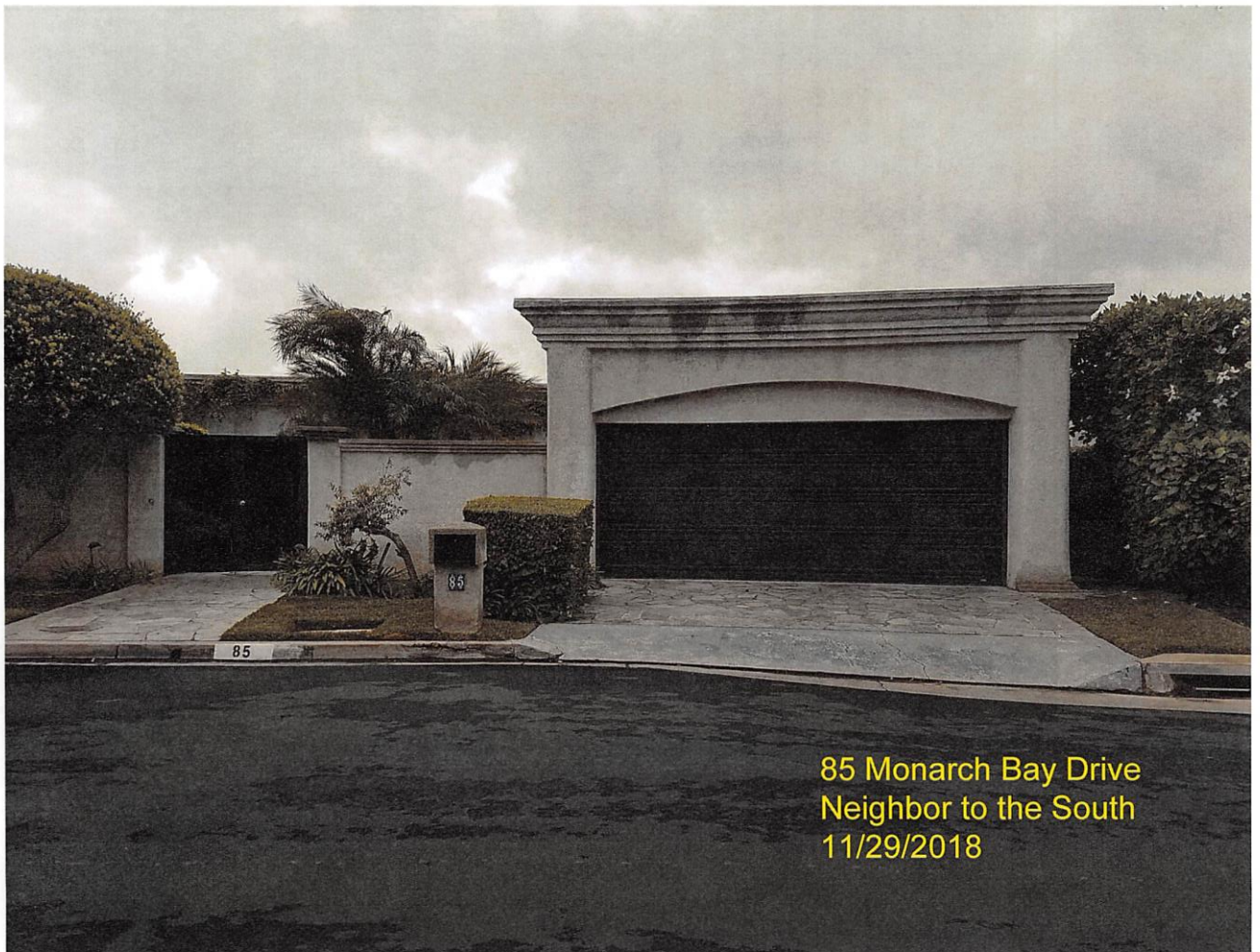
Supporting Document 2: Vicinity Map



Supporting Document 3: Site Photos

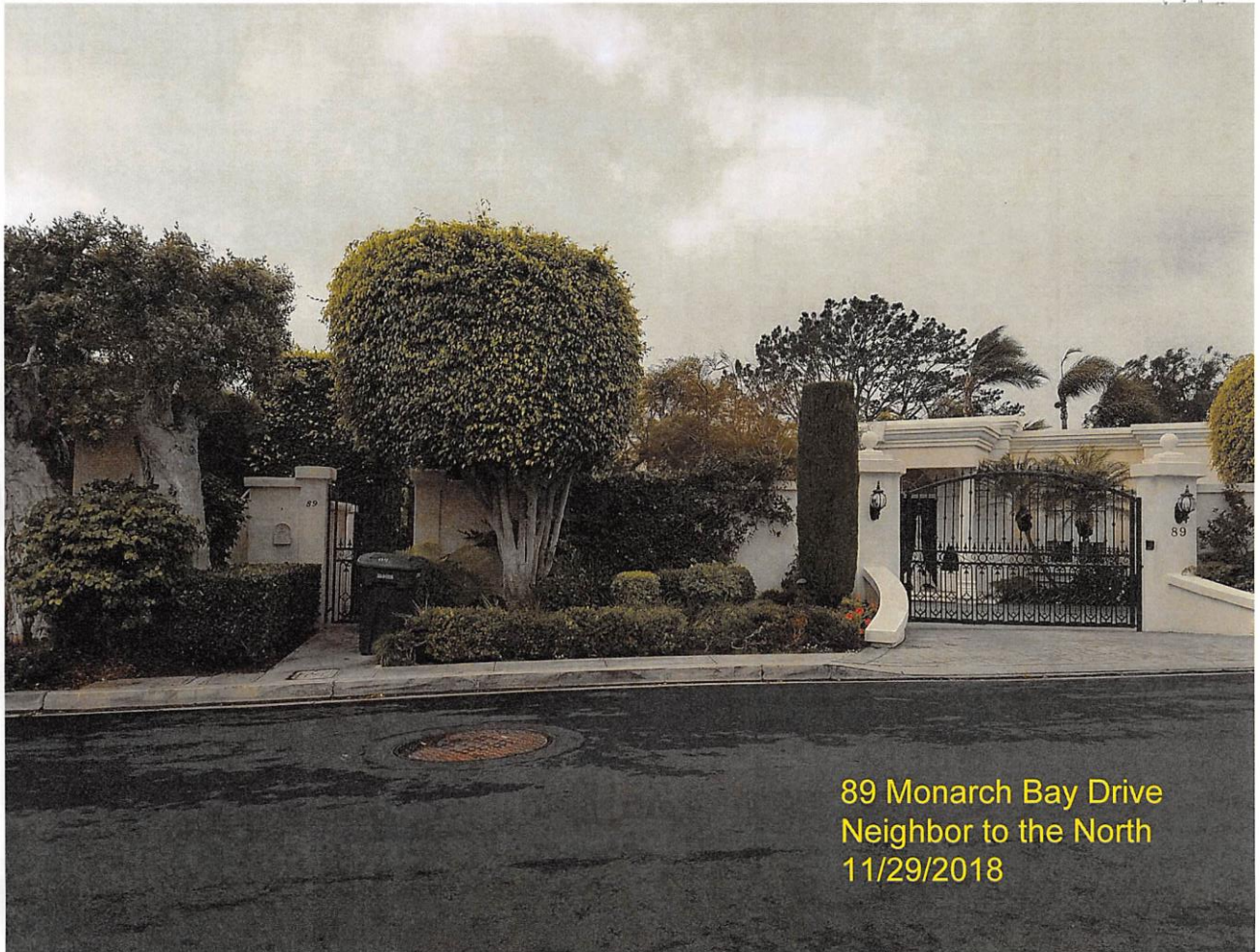








88 Monarch Bay Drive
Neighbor Across the Street
11/29/2018



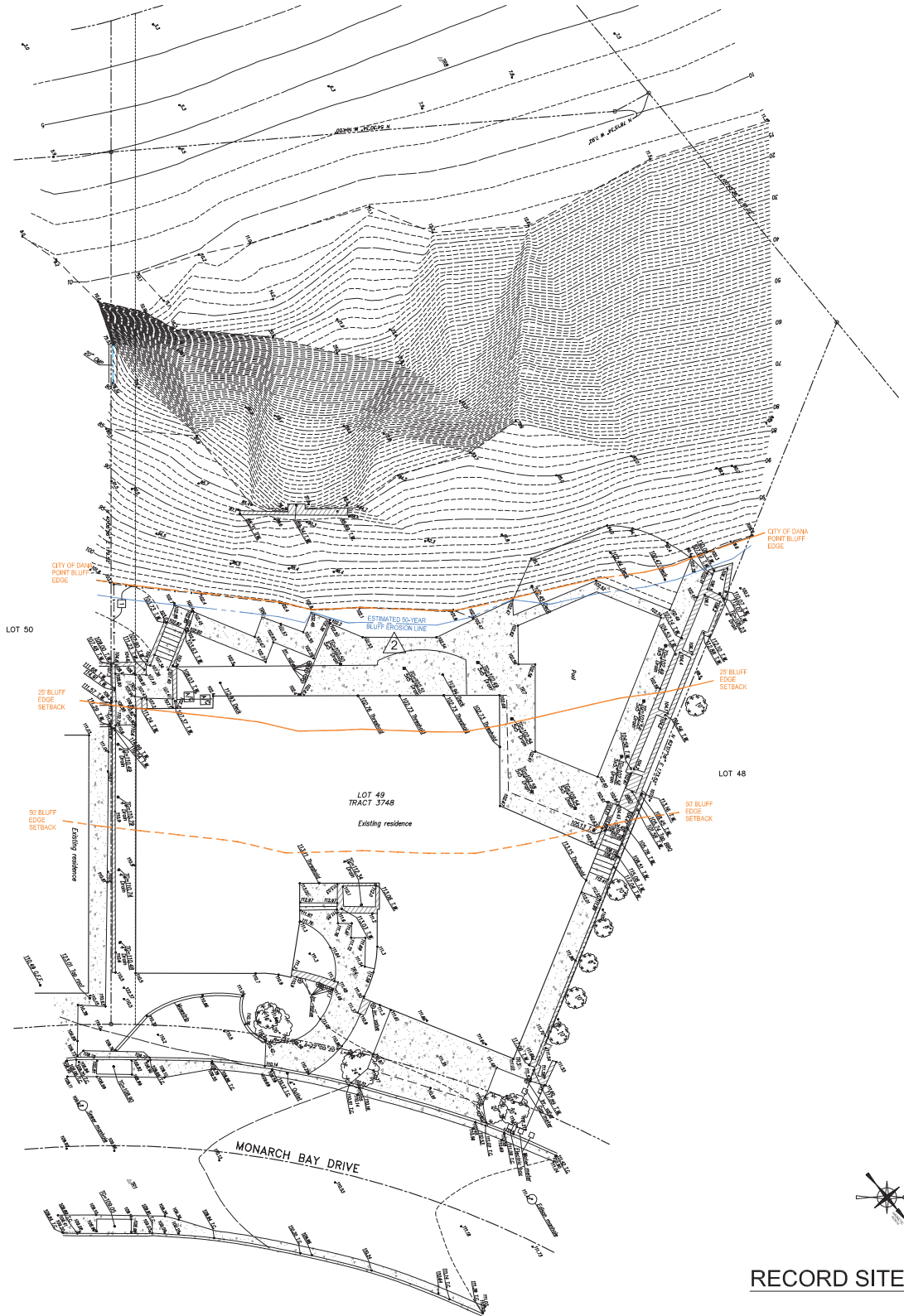
89 Monarch Bay Drive
Neighbor to the North
11/29/2018

Supporting Document 4: Architectural Plans

ATTACHMENT

STILES RESIDENCE REMODEL

OVERALL SITE PLAN



RECORD SITE PLAN
1" = 15'-0"

PROJECT ADDRESS

87 MONARCH BAY DRIVE, DANA POINT, CA 92629

LEGAL DESCRIPTION

A.P.N. 670-121-58 AND 670-121-59.

PARCEL 1:
THE NORTHEASTERLY 160 FEET OF LOTS 49, IN THE CITY OF DANA POINT, COUNTY OF ORANGE, STATE OF CALIFORNIA, AS SHOWN ON A MAP THEREOF RECORDED IN BOOK 142, PAGES 30 TO 34 OF MISCELLANEOUS MAPS, RECORDS OF SAID COUNTY.

PARCEL 2:
LOT 49 OF TRACT NO. 3748, IN THE CITY OF DANA POINT, COUNTY OF ORANGE, STATE OF CALIFORNIA, AS SHOWN ON A MAP THEREOF, RECORDED IN BOOK 142, PAGES 30 TO 34, OF MISCELLANEOUS MAPS, RECORDS OF SAID COUNTY.
EXCEPT THE NORTHEASTERLY 160 FEET OFF LOT 49, SAID DISTANCE BEING MEASURED ON THE NORTH-WESTERLY LINE OF SAID LOT.
ALSO EXCEPT ANY PORTION OF SAID LAND LYING BELOW THE ORDINARY HIGH TIDE LINE OF THE PACIFIC OCEAN.
ALSO EXCEPT ANY PORTION OF SAID LAND LYING OUTSIDE OF THE PATENT LINES OF THE RANCHO NIGUEL, AS SUCH LINES EXISTED AT THE TIME OF THE ISSUANCE OF THE PATENT, WHICH WAS NOT FORMED BY THE DEPOSITS OF ALLUVION FROM NATURAL CAUSES AND BY IMPERCEPTIBLE DEGREES.

PARCEL 3:
NONEXCLUSIVE EASEMENTS FOR INGRESS, EGRESS, ACCESS, MAINTENANCE, REPAIRS, DRAINAGE, ENCROACHMENT, SUPPORT, USE AND ENJOYMENT AS DESCRIBED IN THE "MASTER DECLARATION" OF COVENANTS, CONDITIONS AND RESTRICTIONS AND RESERVATION OF EASEMENTS FOR "MONARCH BAY" RECORDED ON DECEMBER 19, 2012 AS INSTRUMENT NO.2012000786869, AS SAME MAY BE RESTATED AND / OR AMENDED FROM TIME TO TIME (THE "MASTER DECLARATION"), IN THE OFFICIAL RECORDS OF ORANGE COUNTY, CALIFORNIA.

PROJECT INFORMATION

TYPE OF CONSTRUCTION: V-B

TYPE OF OCCUPANCY: R-3 / U

APPLICABLE CODES: 2016 CALIFORNIA RESIDENTIAL CODE (CRC), CALIFORNIA BUILDING CODE (CBC), MECHANICAL (CMC), PLUMBING (CPC), ELECTRICAL (CEC), ENERGY (CENC), FIRE (CFC), GREEN (CGBC), DANA POINT MUNICIPAL CODE (DPMC), MONARCH BAY ASSOCIATION.

SPRINKLERS REQUIRED: NO

PROJECT SUMMARY TABLES

PROJECT DATA					
DESCRIPTION		EXISTING	ADDITION / (REDUCTION)	PROPOSED TOTAL	REMODEL
LIVING AREA:	LOWER FLR.	1,734.5 SF	+121 / (-0)	1,855.5 SF	0 SF
	UPPER FLR.	4,129.0 SF	+0 / (-0)	4,129.0 SF	0 SF
STORAGE:	LOWER FLR.	112.0 SF	+61 / (-36)	137.0 SF	0 SF
	UPPER FLR.	0 SF	+0 / (-0)	0 SF	0 SF
TOTAL HABITABLE		5,975.5 SF	+182 / (-36)	6,121.5 SF	0 SF
GARAGE:	UPPER FLR.	700 SF	+0 / (-0)	700 SF	0 SF
TOTAL ELEVATED DECK:		351 SF	+0 / (-0)	351 SF	0 SF
TOTAL MECHANICAL:		128 SF	+0 / (-0)	128 SF	0 SF

SCOPE OF WORK

- ELEVATOR ADDITION BETWEEN THE LOWER AND UPPER FLOOR WHICH INCREASES THE INTERIOR LOWER FLOOR AREA BY 146 SQ.FT.
- WINDOW ADDITION IN TWO EXISTING WALLS.
- UPGRADE OF THE HOME LIGHTING AND CONTROL SYSTEM.

CONTACTS

OWNER:
MR & MRS STILES
CONTACT: ARCHITECT
87 MONARCH BAY DRIVE
DANA POINT, CA 92629

ARCHITECT:
MORRIS SKENDERIAN & ASSOC.
CONTACT: MORRIS SKENDERIAN
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STRUCTURAL ENGINEER:
PETRA STRUCTURAL ENGINEERS
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TEL.: 949-748-7170
FAX:
EMAIL: PSARKIS@PETRASTRUCTURAL.COM

ENERGY COMPLIANCE:
MAURER ASSOCIATES
CONTACT: RICK MAURER
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ORANGE, CA. 92669
TEL.: 714-771-1507
FAX: 714-771-2939
EMAIL: T-24@SOCAL.RR.COM

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CONTACT: CHRIS GALLO
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LAGUNA BEACH, CA. 92651
TEL.: 949-715-9874
FAX: 949-715-9875
EMAIL: CHRIS@GALLOBUILDERSINC.COM

SHEET INDEX

ARCHITECTURE

T-1	TITLE SHEET , OVERALL SITE PLAN
R-0.0	OVERALL RECORD SITE PLAN
R-0.1	PARTIAL RECORD SITE PLAN ROTATED
R-2.0	RECORD LOWER LEVEL FLOOR PLAN
R-2.1	RECORD UPPER LEVEL FLOOR PLAN
R-3.0	RECORD EXTERIOR ELEVATIONS
R-3.1	RECORD EXTERIOR ELEVATIONS
R-4.0	RECORD PARTIAL BUILDING SECTIONS
A-2.0	LOWER LEVEL FLOOR PLAN
A-2.1	UPPER LEVEL FLOOR PLAN
A-2.2	ROOF PLAN
A-3.0	EXTERIOR ELEVATIONS
A-3.1	EXTERIOR ELEVATIONS
A-4.0	PARTIAL BUILDING SECTIONS
A-5.0	STORAGE AND POWDER ROOMS INTERIOR ELEV.

CIVIL

1 of 1 TOPOGRAPHIC & BLUFF EDGE SURVEY

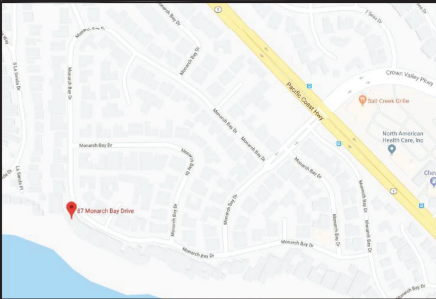
STRUCTURAL

S-0.1	GENERAL NOTES
S-0.2	ABBREVIATIONS
S-2.0	LOWER FLOOR PLAN
S-2.1	UPPER FLOOR PLAN
S-3.0	TYPICAL CONCRETE DETAILS
S-3.1	CONCRETE DETAILS
S-6.0	TYPICAL WOOD DETAILS

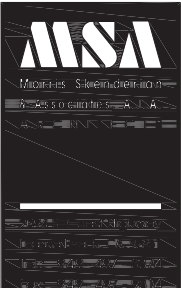
REVISIONS

1	1 / 25 / 2019	2ND CITY PLANNING REVIEW
2	2 / 20 / 2019	PUBLIC WORKS COMMENTS

VICINITY MAP



REVISIONS		
NO.	DATE	BY
1	1/25/2019	DJB
2	2/20/2019	DJB

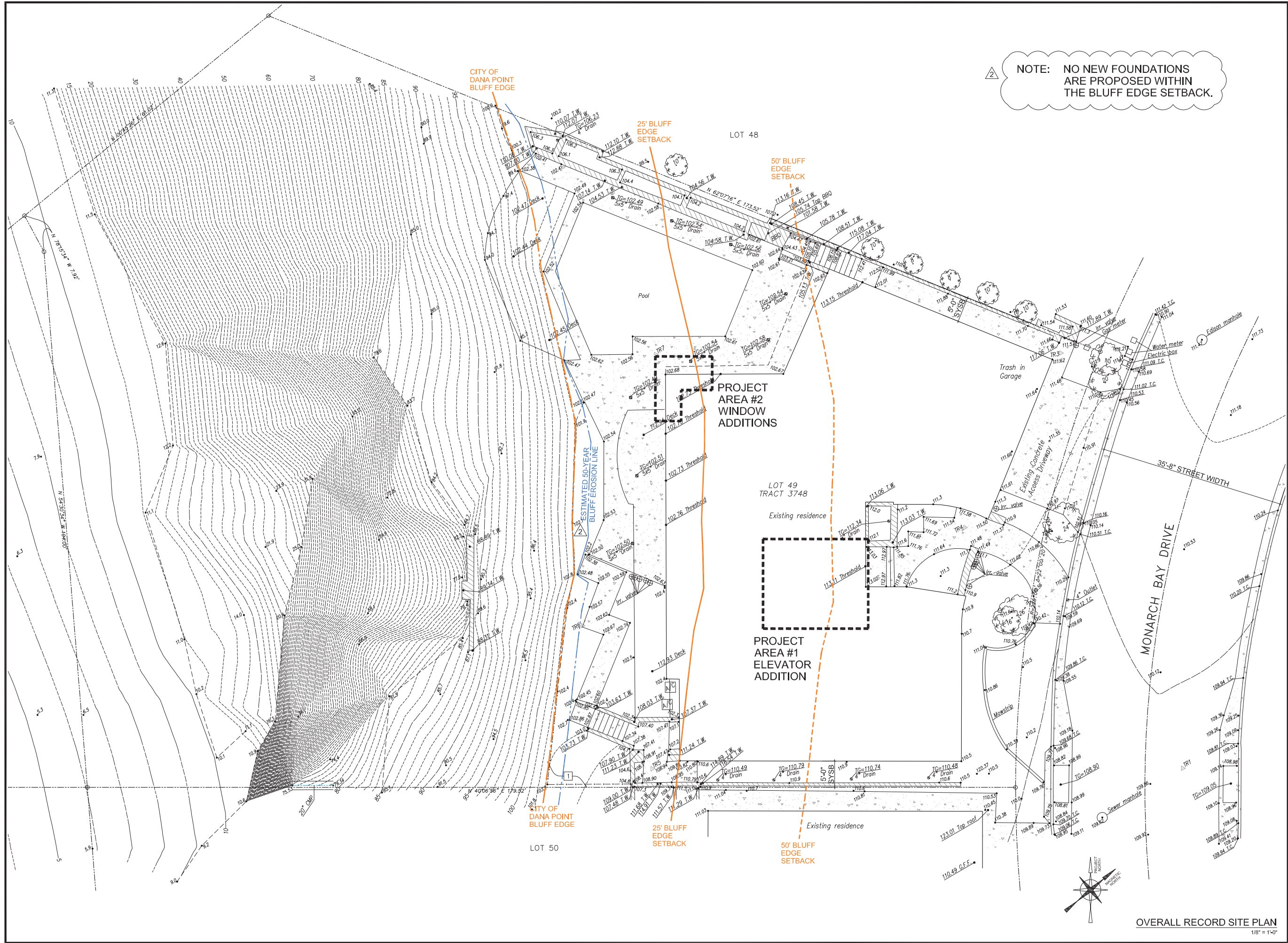


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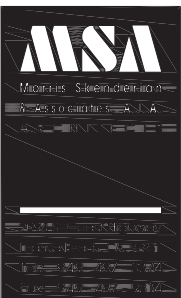
TITLE SHEET
STILES RESIDENCE REMODEL
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

REVIEWED BY / DATE	
-	
FIRST SUBMITTAL DATE 10/11/2018	
DRAWN BY	JOB NUMBER 18-011
SHEET NUMBER T-1	



NOTE: NO NEW FOUNDATIONS ARE PROPOSED WITHIN THE BLUFF EDGE SETBACK.

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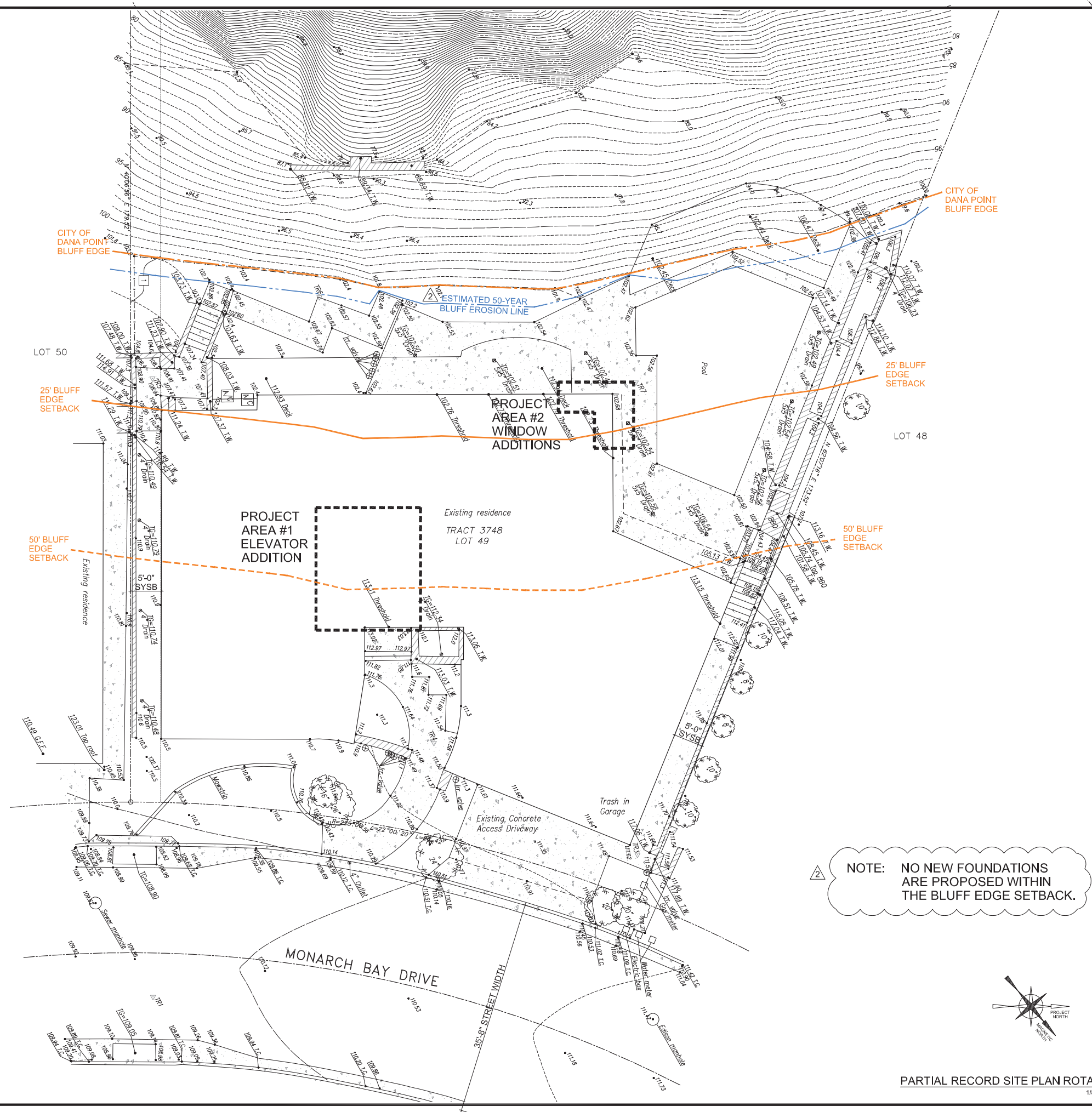


OVERALL RECORD SITE PLAN

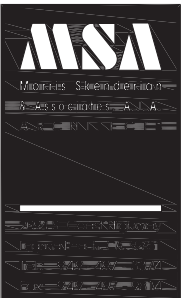
STILES RESIDENCE REMODEL

87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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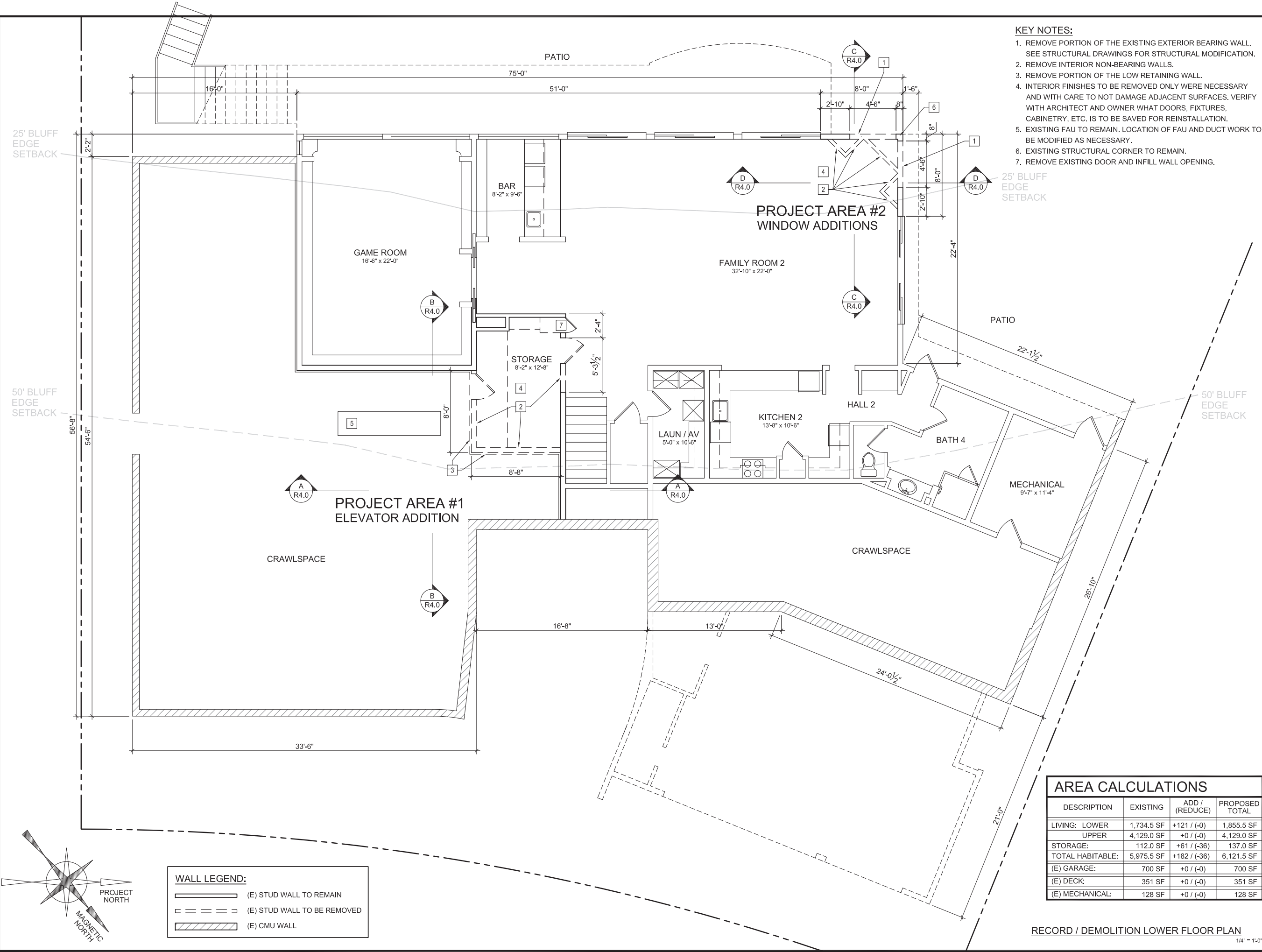


PARTIAL RECORD SITE PLAN ROTATED

STILES RESIDENCE REMODEL

87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

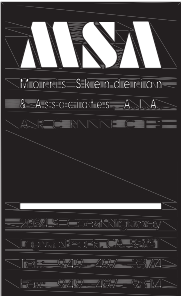
REVIEWED BY / DATE	
FIRST SUBMITTAL DATE	
10/11/2018	
DRAWN BY	JOB NUMBER
	18-011
SHEET NUMBER	
R-0.1	
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- KEY NOTES:**
1. REMOVE PORTION OF THE EXISTING EXTERIOR BEARING WALL. SEE STRUCTURAL DRAWINGS FOR STRUCTURAL MODIFICATION.
 2. REMOVE INTERIOR NON-BEARING WALLS.
 3. REMOVE PORTION OF THE LOW RETAINING WALL.
 4. INTERIOR FINISHES TO BE REMOVED ONLY WERE NECESSARY AND WITH CARE TO NOT DAMAGE ADJACENT SURFACES. VERIFY WITH ARCHITECT AND OWNER WHAT DOORS, FIXTURES, CABINETRY, ETC. IS TO BE SAVED FOR REINSTALLATION.
 5. EXISTING FAU TO REMAIN. LOCATION OF FAU AND DUCT WORK TO BE MODIFIED AS NECESSARY.
 6. EXISTING STRUCTURAL CORNER TO REMAIN.
 7. REMOVE EXISTING DOOR AND INFILL WALL OPENING.

AREA CALCULATIONS			
DESCRIPTION	EXISTING	ADD / (REDUCE)	PROPOSED TOTAL
LIVING: LOWER	1,734.5 SF	+121 / (-0)	1,855.5 SF
UPPER	4,129.0 SF	+0 / (-0)	4,129.0 SF
STORAGE:	112.0 SF	+61 / (-36)	137.0 SF
TOTAL HABITABLE:	5,975.5 SF	+182 / (-36)	6,121.5 SF
(E) GARAGE:	700 SF	+0 / (-0)	700 SF
(E) DECK:	351 SF	+0 / (-0)	351 SF
(E) MECHANICAL:	128 SF	+0 / (-0)	128 SF

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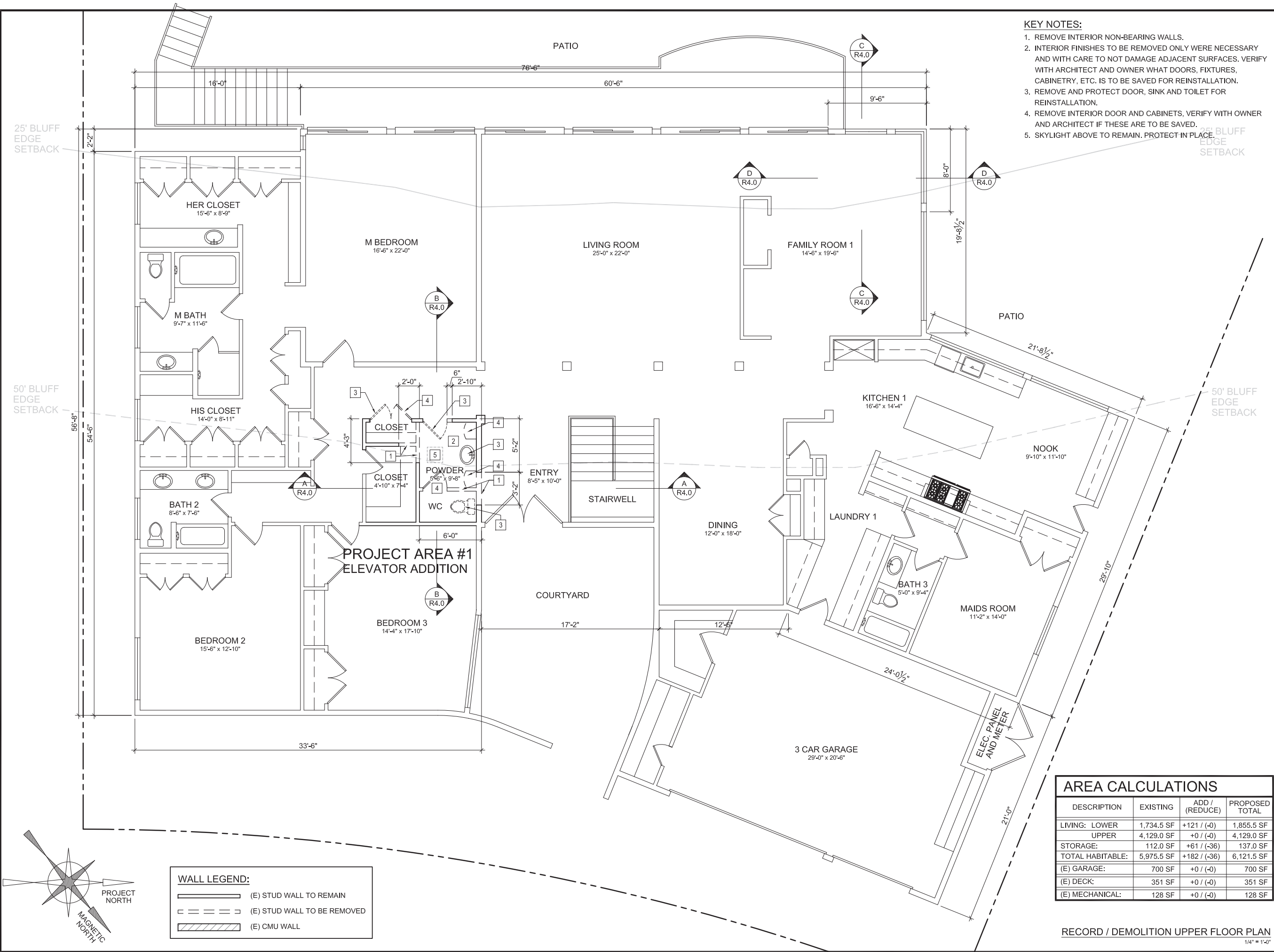
RECORD / DEMOLITION LOWER FLOOR PLAN

STILES RESIDENCE REMODEL

87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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FIRST SUBMITTAL DATE	
10/11/2018	
DRAWN BY	JOB NUMBER
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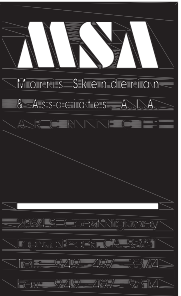
RECORD / DEMOLITION LOWER FLOOR PLAN
1/4" = 1'-0"



- KEY NOTES:**
1. REMOVE INTERIOR NON-BEARING WALLS.
 2. INTERIOR FINISHES TO BE REMOVED ONLY WERE NECESSARY AND WITH CARE TO NOT DAMAGE ADJACENT SURFACES. VERIFY WITH ARCHITECT AND OWNER WHAT DOORS, FIXTURES, CABINETS, ETC. IS TO BE SAVED FOR REINSTALLATION.
 3. REMOVE AND PROTECT DOOR, SINK AND TOILET FOR REINSTALLATION.
 4. REMOVE INTERIOR DOOR AND CABINETS, VERIFY WITH OWNER AND ARCHITECT IF THESE ARE TO BE SAVED.
 5. SKYLIGHT ABOVE TO REMAIN. PROTECT IN PLACE.

AREA CALCULATIONS			
DESCRIPTION	EXISTING	ADD / (REDUCE)	PROPOSED TOTAL
LIVING: LOWER	1,734.5 SF	+121 / (-0)	1,855.5 SF
UPPER	4,129.0 SF	+0 / (-0)	4,129.0 SF
STORAGE:	112.0 SF	+61 / (-36)	137.0 SF
TOTAL HABITABLE:	5,975.5 SF	+182 / (-36)	6,121.5 SF
(E) GARAGE:	700 SF	+0 / (-0)	700 SF
(E) DECK:	351 SF	+0 / (-0)	351 SF
(E) MECHANICAL:	128 SF	+0 / (-0)	128 SF

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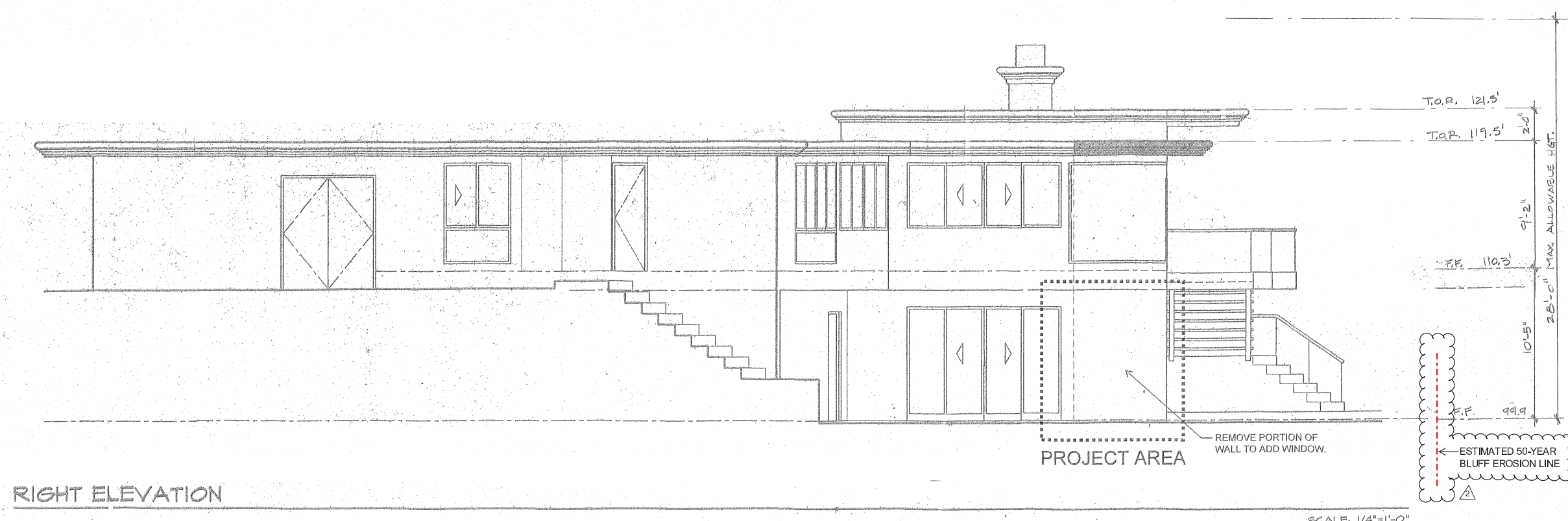


RECORD / DEMOLITION UPPER FLOOR PLAN

STILES RESIDENCE REMODEL

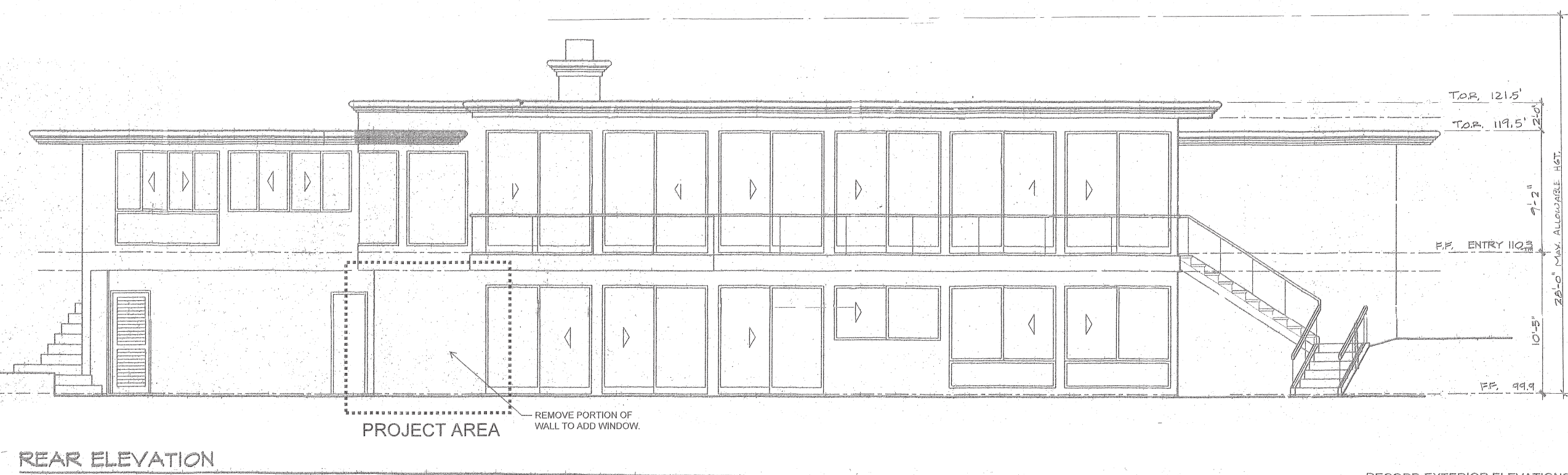
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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R-2.1	
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RIGHT ELEVATION

SCALE: 1/4"=1'-0"



REAR ELEVATION

SCALE: 1/4"=1'-0"

RECORD EXTERIOR ELEVATIONS
1/4" = 1'-0"

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MSA
Morris Skenderian & Associates, A.I.A.
ARCHITECTS

2094 S. Coast Highway
Laguna Beach, CA 92651
Tel: 949-497-3374
Fax: 949-497-9814

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LICENSED ARCHITECT
MORRIS M. SKENDERIAN
NO. C10780
EX. 13121
STATE OF CALIFORNIA

RECORD EXTERIOR ELEVATIONS

STILES RESIDENCE REMODEL
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

REVIEWED BY / DATE	
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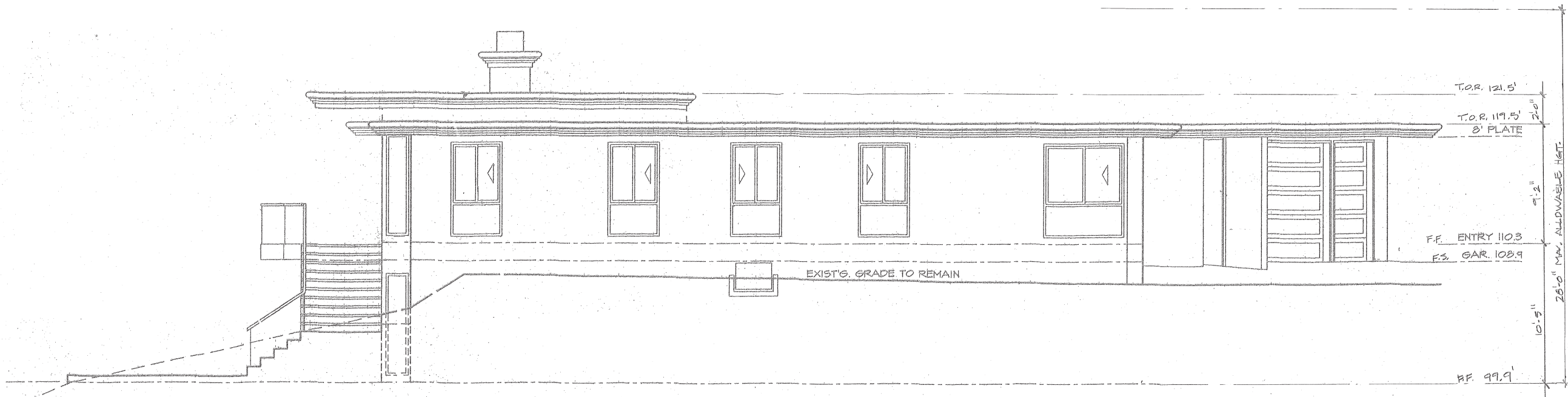
LICENSED ARCHITECT
MORRIS M. SKENDERIAN
NO. C 9793
REL. 10-21
STATE OF CALIFORNIA

RECORD EXTERIOR ELEVATIONS

STILES RESIDENCE REMODEL

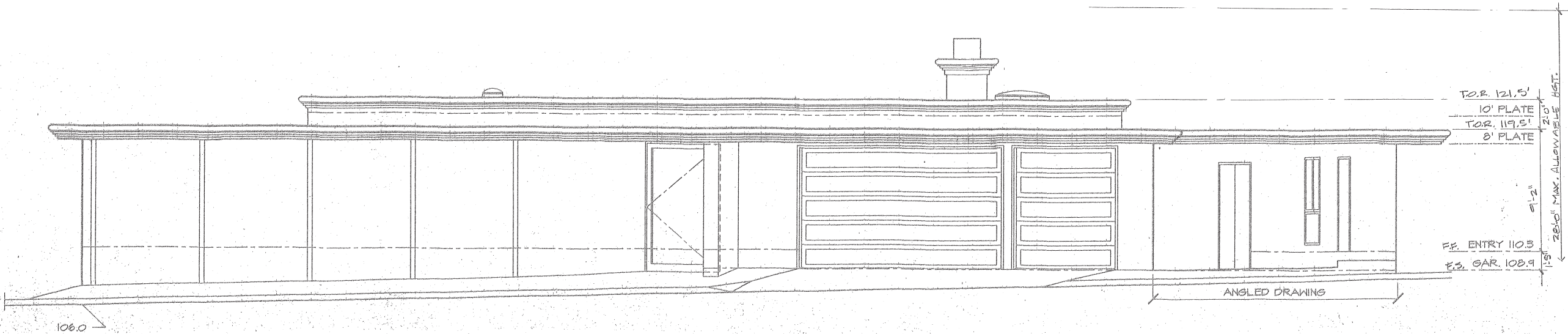
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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FIRST SUBMITTAL DATE 10/11/2018	
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SHEET NUMBER R-3.1	



LEFT ELEVATION NOTE: NO WORK THIS ELEVATION

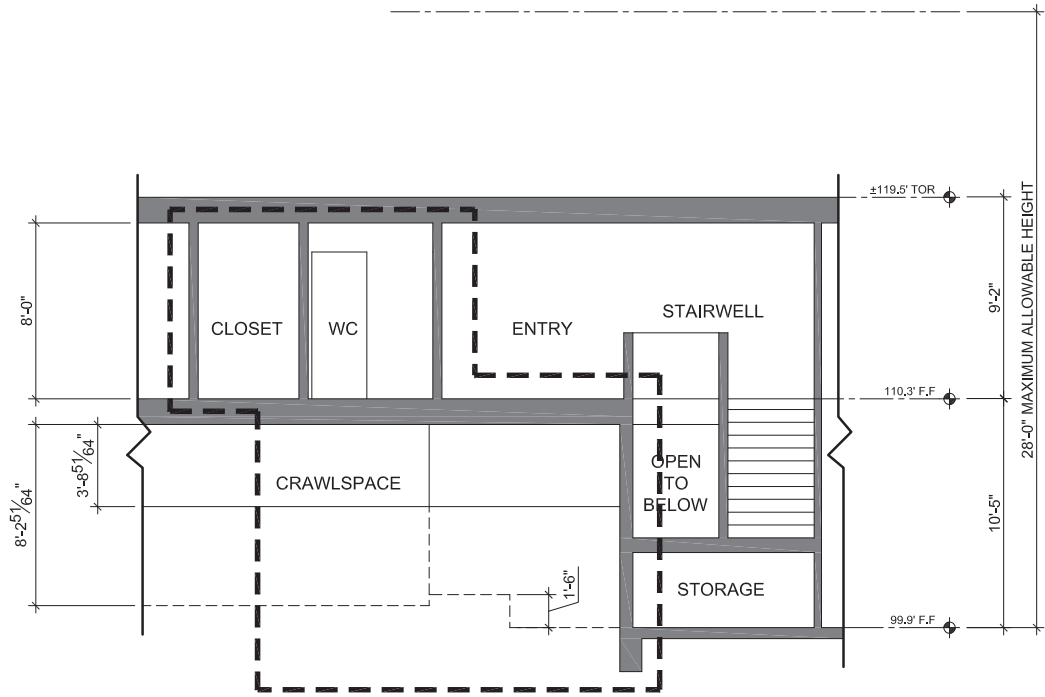
SCALE: 1/4"=1'-0"



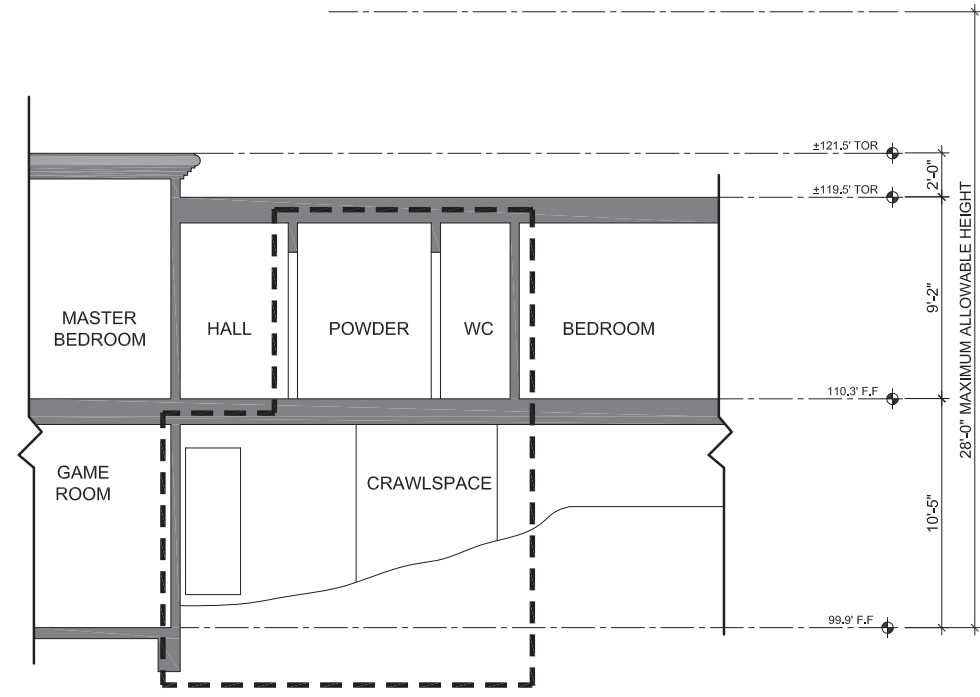
FRONT ELEVATION NOTE: NO WORK THIS ELEVATION

SCALE: 1/4"=1'-0"

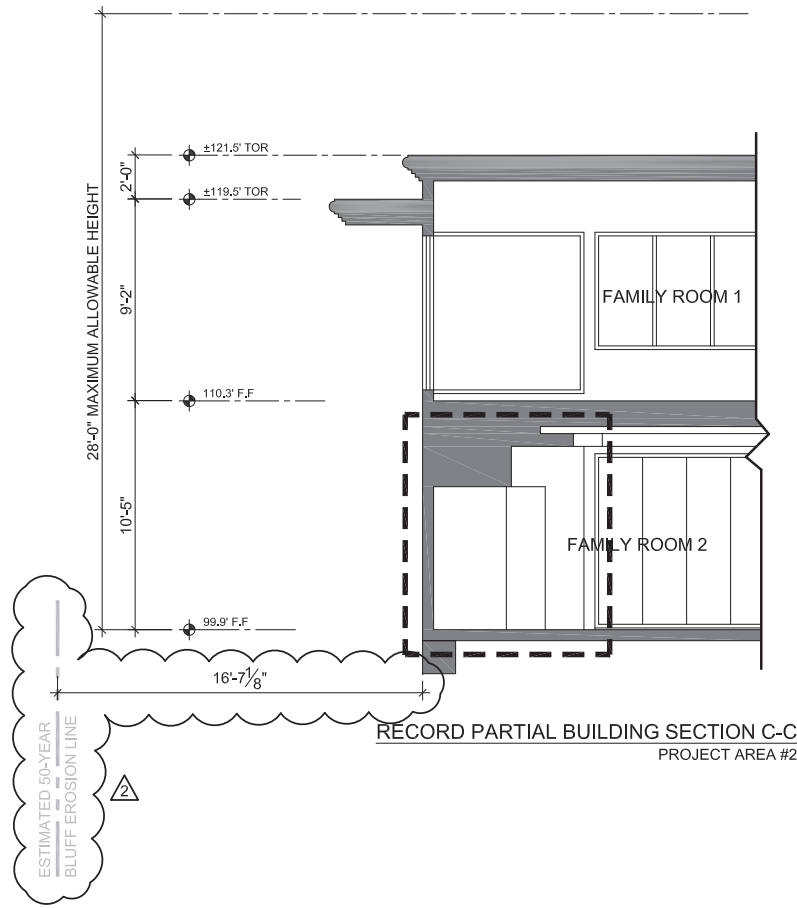
RECORD EXTERIOR ELEVATIONS
1/4" = 1'-0"



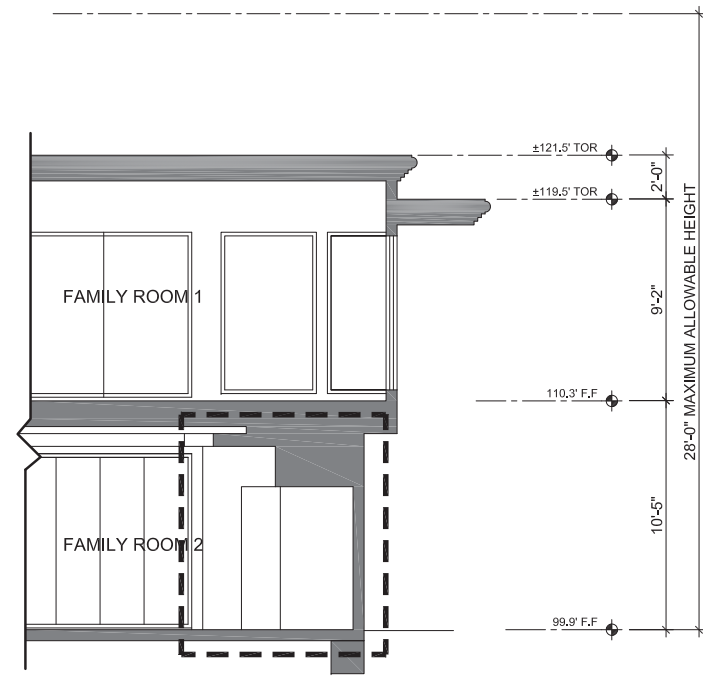
RECORD PARTIAL BUILDING SECTION A-A
PROJECT AREA #1



RECORD PARTIAL BUILDING SECTION B-B
PROJECT AREA #1



RECORD PARTIAL BUILDING SECTION C-C
PROJECT AREA #2

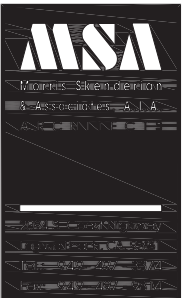


RECORD PARTIAL BUILDING SECTION D-D
PROJECT AREA #2

INDICATES
PROJECT
AREAS

PARTIAL RECORD / DEMOLITION BUILDING SECTIONS
1/4" = 1'-0"

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NO.	DATE	BY
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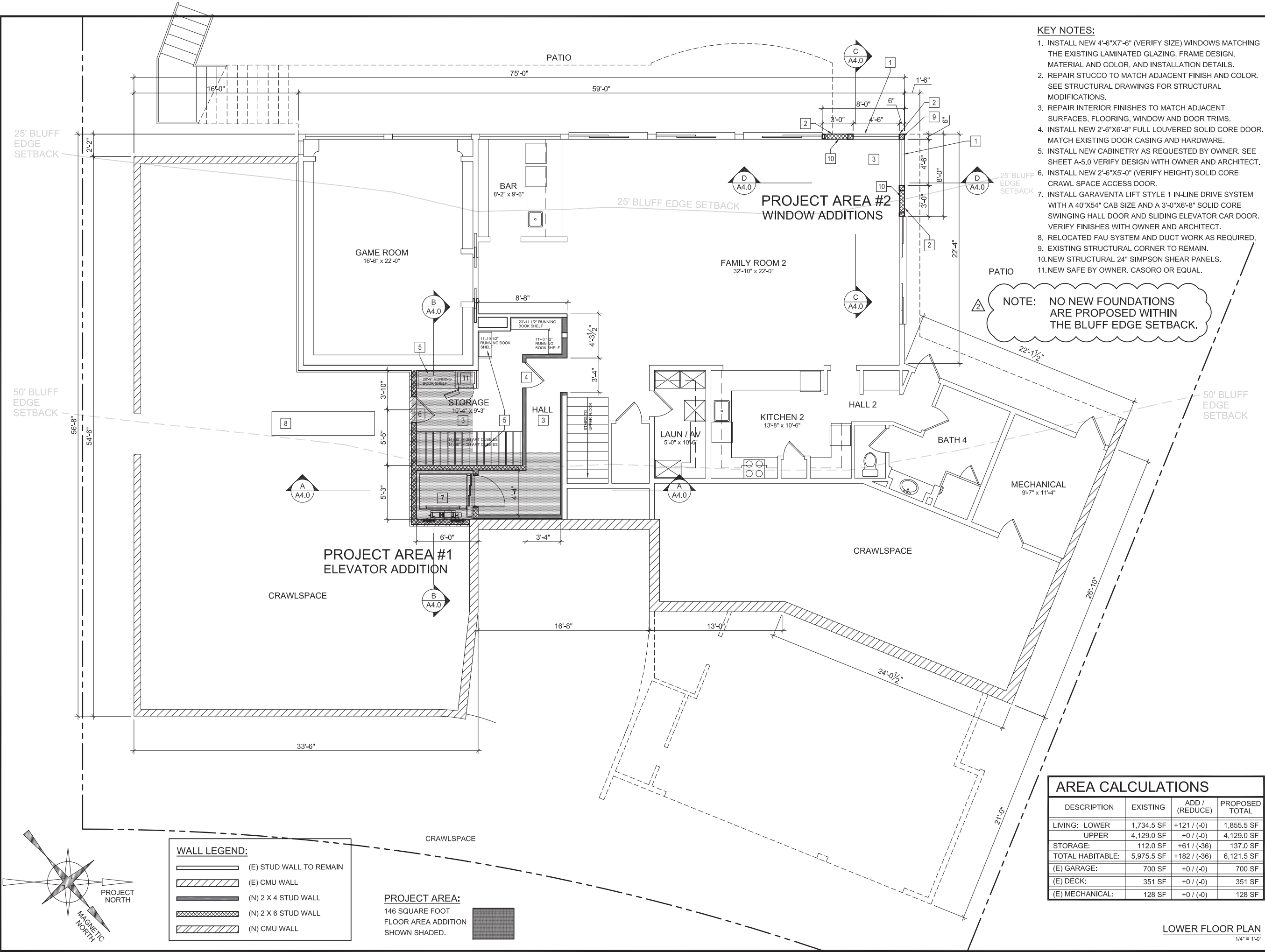


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RECORD PARTIAL BUILDING SECTIONS
STILES RESIDENCE REMODEL
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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10/11/2018	
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18-011	18-011
SHEET NUMBER	
R-4.0	

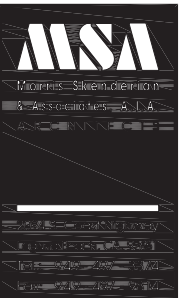


- KEY NOTES:**
1. INSTALL NEW 4'-6"x7'-6" (VERIFY SIZE) WINDOWS MATCHING THE EXISTING LAMINATED GLAZING, FRAME DESIGN, MATERIAL AND COLOR, AND INSTALLATION DETAILS.
 2. REPAIR STUCCO TO MATCH ADJACENT FINISH AND COLOR. SEE STRUCTURAL DRAWINGS FOR STRUCTURAL MODIFICATIONS.
 3. REPAIR INTERIOR FINISHES TO MATCH ADJACENT SURFACES, FLOORING, WINDOW AND DOOR TRIMS.
 4. INSTALL NEW 2'-6"x6'-8" FULL LOUVERED SOLID CORE DOOR. MATCH EXISTING DOOR CASING AND HARDWARE.
 5. INSTALL NEW CABINETY AS REQUESTED BY OWNER. SEE SHEET A-5.0 VERIFY DESIGN WITH OWNER AND ARCHITECT.
 6. INSTALL NEW 2'-6"x5'-0" (VERIFY HEIGHT) SOLID CORE CRAWL SPACE ACCESS DOOR.
 7. INSTALL GARAVENTA LIFT STYLE 1 IN-LINE DRIVE SYSTEM WITH A 40"x54" CAB SIZE AND A 3'-0"x6'-8" SOLID CORE SWINGING HALL DOOR AND SLIDING ELEVATOR CAR DOOR. VERIFY FINISHES WITH OWNER AND ARCHITECT.
 8. RELOCATED FAU SYSTEM AND DUCT WORK AS REQUIRED.
 9. EXISTING STRUCTURAL CORNER TO REMAIN.
 10. NEW STRUCTURAL 24" SIMPSON SHEAR PANELS.
 11. NEW SAFE BY OWNER. CASORO OR EQUAL.

NOTE: NO NEW FOUNDATIONS ARE PROPOSED WITHIN THE BLUFF EDGE SETBACK.

AREA CALCULATIONS			
DESCRIPTION	EXISTING	ADD / (REDUCE)	PROPOSED TOTAL
LIVING: LOWER	1,734.5 SF	+121 / (-0)	1,855.5 SF
UPPER	4,129.0 SF	+0 / (-0)	4,129.0 SF
STORAGE:	112.0 SF	+61 / (-36)	137.0 SF
TOTAL HABITABLE:	5,975.5 SF	+182 / (-36)	6,121.5 SF
(E) GARAGE:	700 SF	+0 / (-0)	700 SF
(E) DECK:	351 SF	+0 / (-0)	351 SF
(E) MECHANICAL:	128 SF	+0 / (-0)	128 SF

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NO.	DATE	BY
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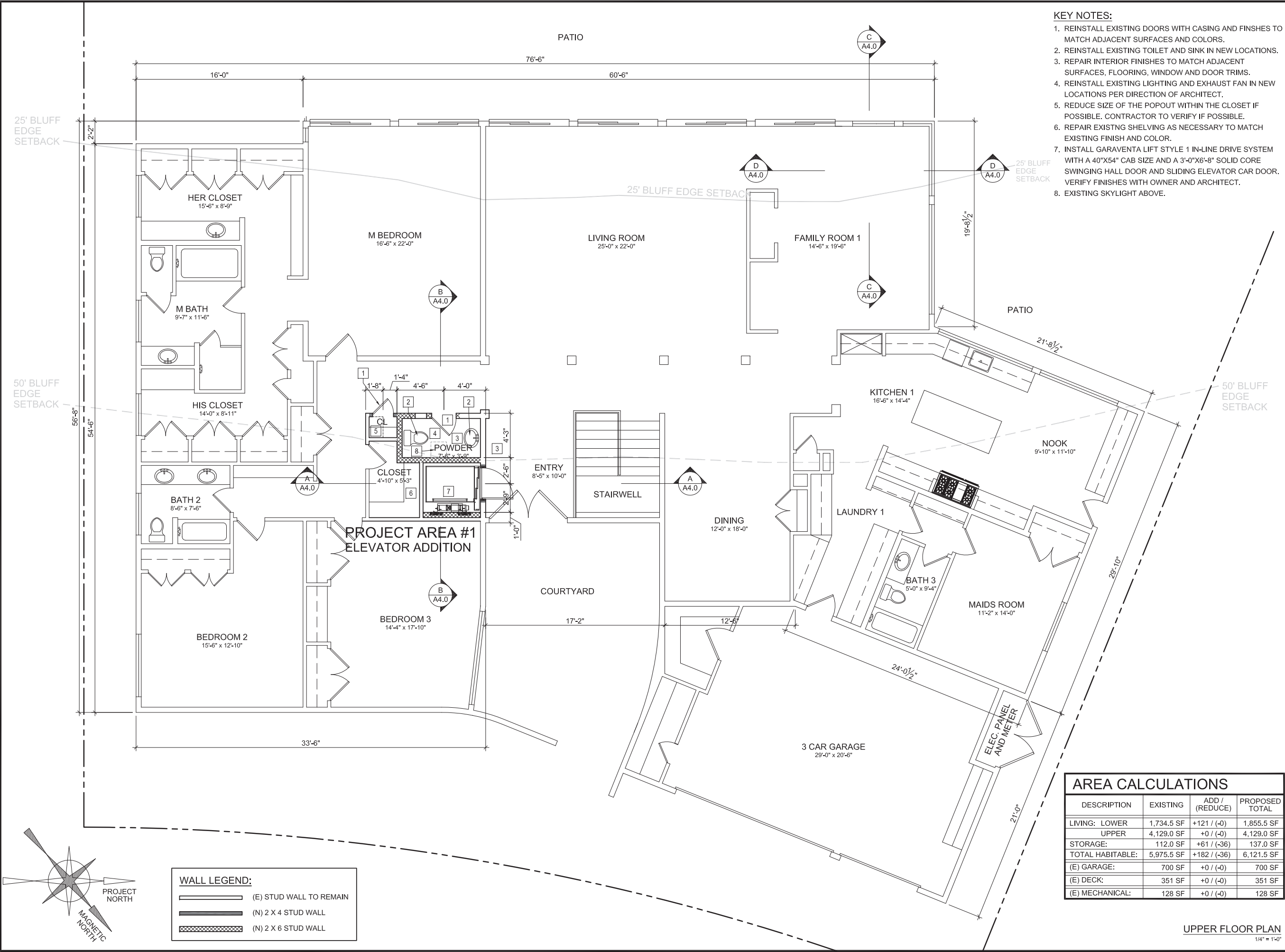
LOWER FLOOR PLAN

STILES RESIDENCE REMODEL

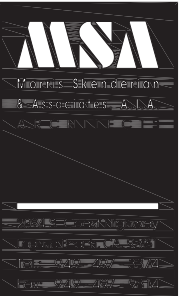
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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LOWER FLOOR PLAN
1/4" = 1'-0"



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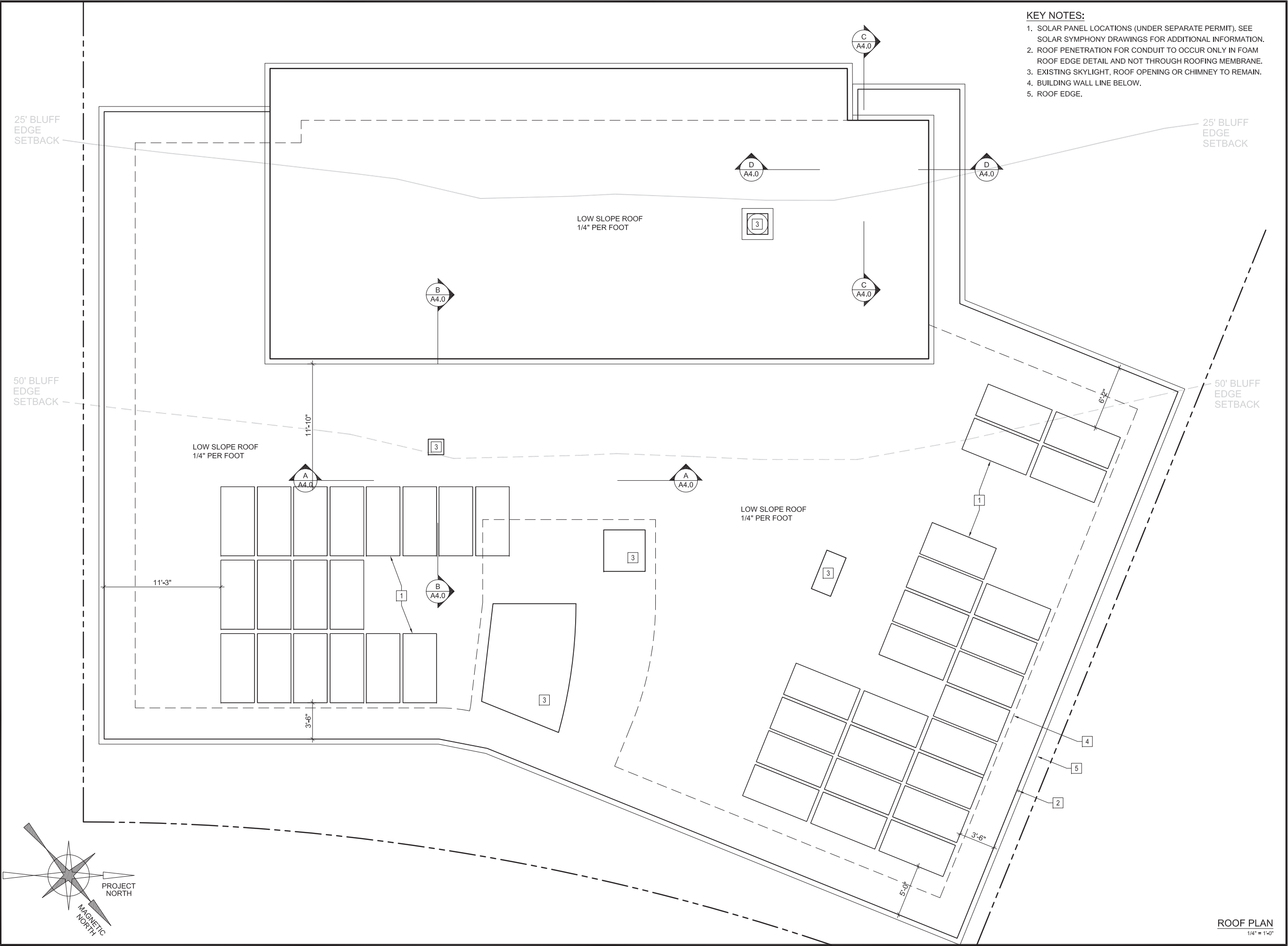


UPPER FLOOR PLAN

STILES RESIDENCE REMODEL

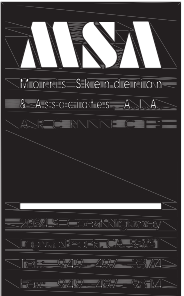
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SHEET NUMBER	
A-2.1	
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- KEY NOTES:
1. SOLAR PANEL LOCATIONS (UNDER SEPARATE PERMIT). SEE SOLAR SYMPHONY DRAWINGS FOR ADDITIONAL INFORMATION.
 2. ROOF PENETRATION FOR CONDUIT TO OCCUR ONLY IN FOAM ROOF EDGE DETAIL AND NOT THROUGH ROOFING MEMBRANE.
 3. EXISTING SKYLIGHT, ROOF OPENING OR CHIMNEY TO REMAIN.
 4. BUILDING WALL LINE BELOW.
 5. ROOF EDGE.

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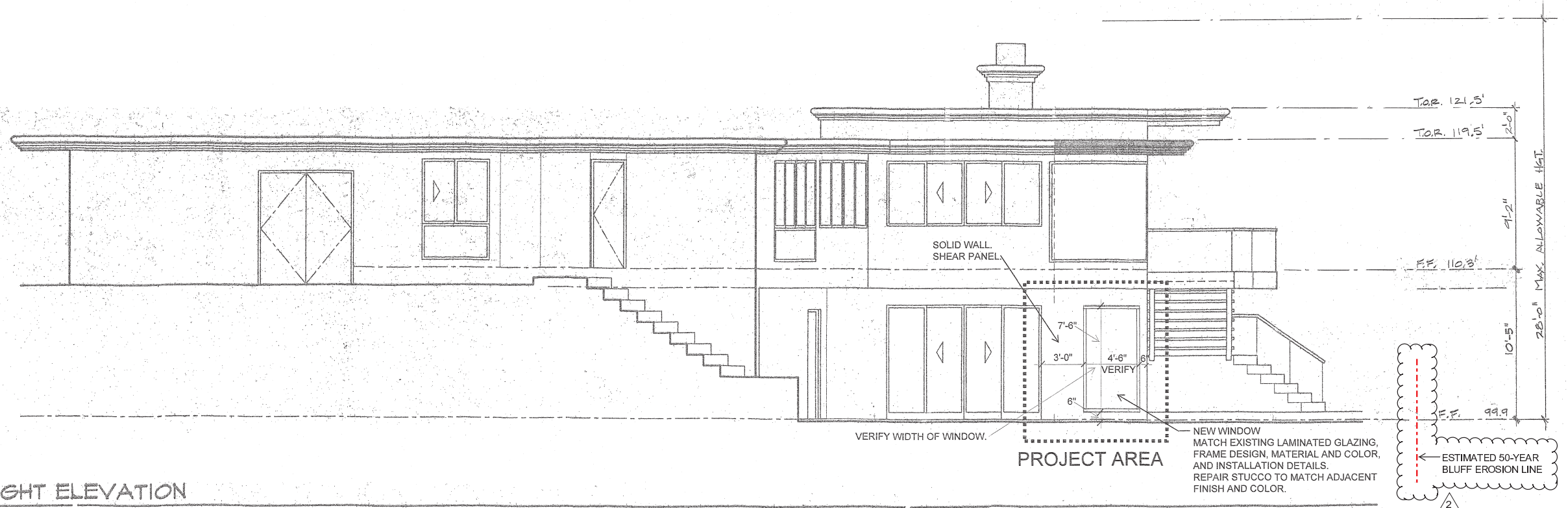


ROOF PLAN

STILES RESIDENCE REMODEL

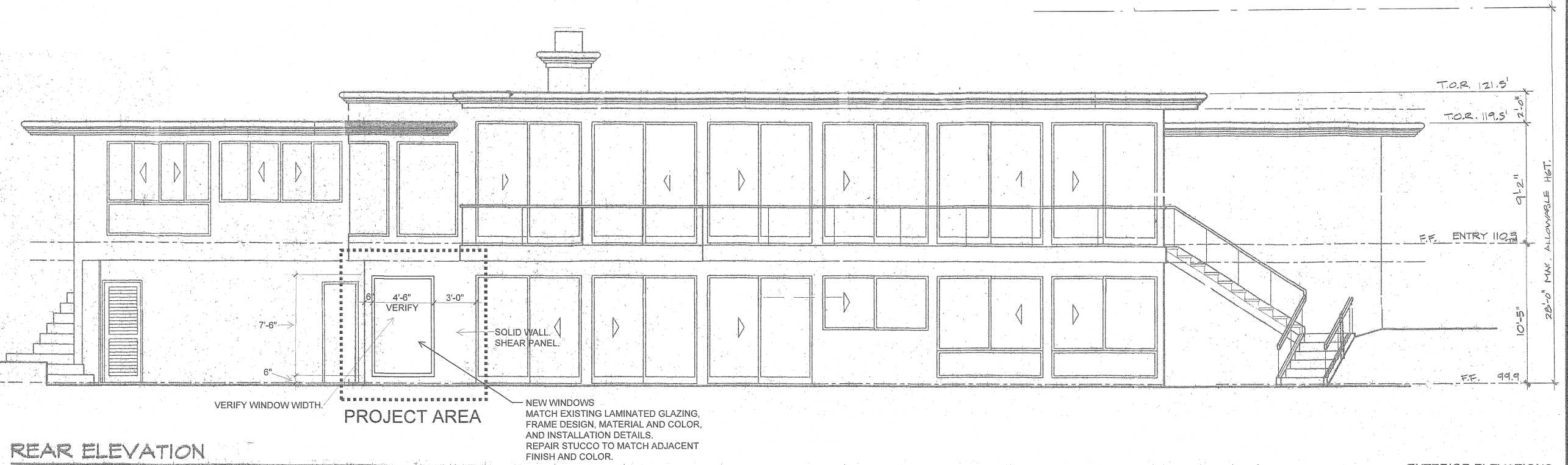
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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-	
FIRST SUBMITTAL DATE	
10/11/2018	
DRAWN BY	JOB NUMBER
18-011	18-011
SHEET NUMBER	
A-2.2	
00	



RIGHT ELEVATION

SCALE: 1/4"=1'-0"



REAR ELEVATION

SCALE: 1/4"=1'-0"

EXTERIOR ELEVATIONS
1/4" = 1'-0"

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2	2/20/2019	DJB



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EXTERIOR ELEVATIONS
STILES RESIDENCE REMODEL
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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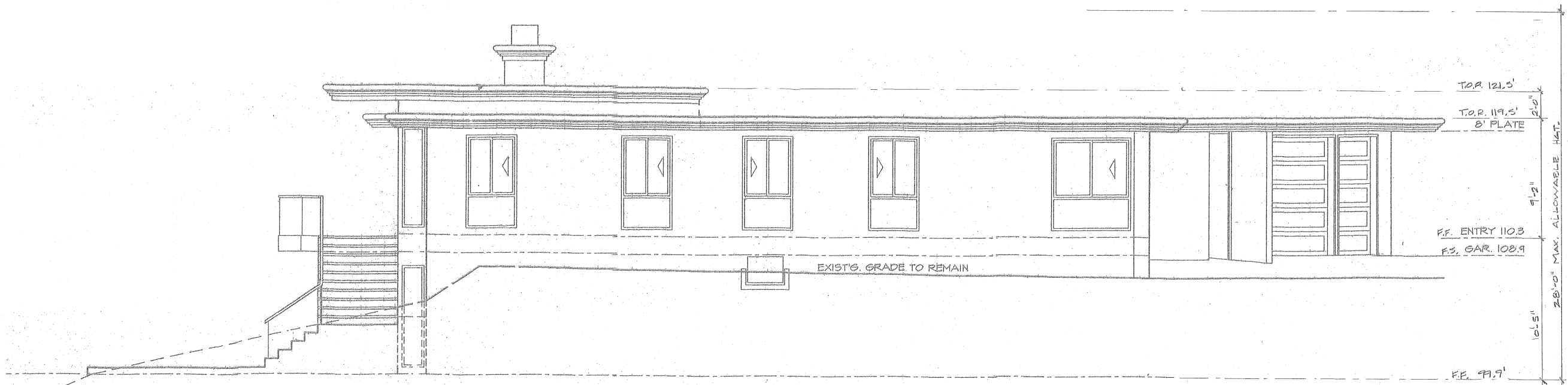


EXTERIOR ELEVATIONS

STILES RESIDENCE REMODEL

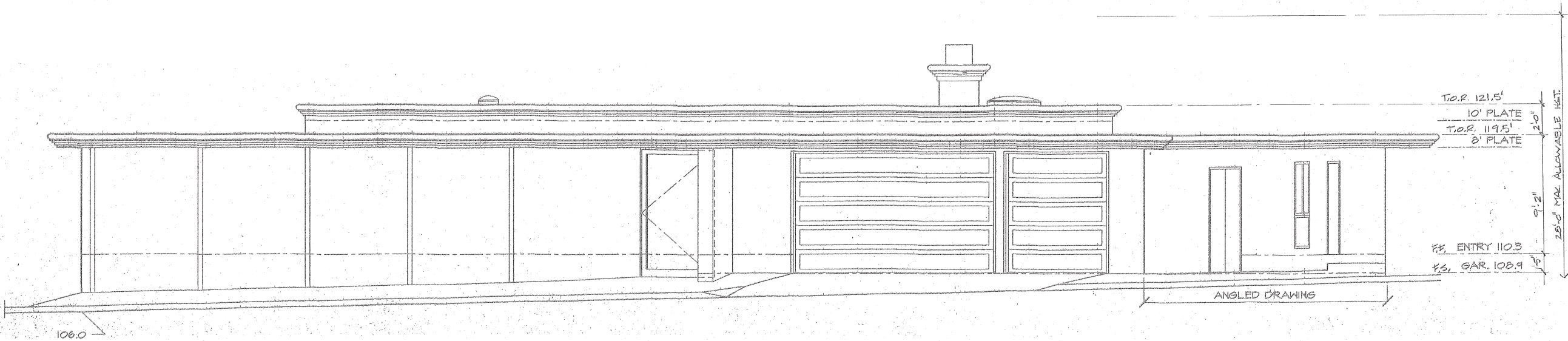
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

REVIEWED BY / DATE	
FIRST SUBMITTAL DATE	
10/11/2018	
DRAWN BY	JOB NUMBER
18-011	
SHEET NUMBER	
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LEFT ELEVATION NOTE: NO WORK THIS ELEVATION

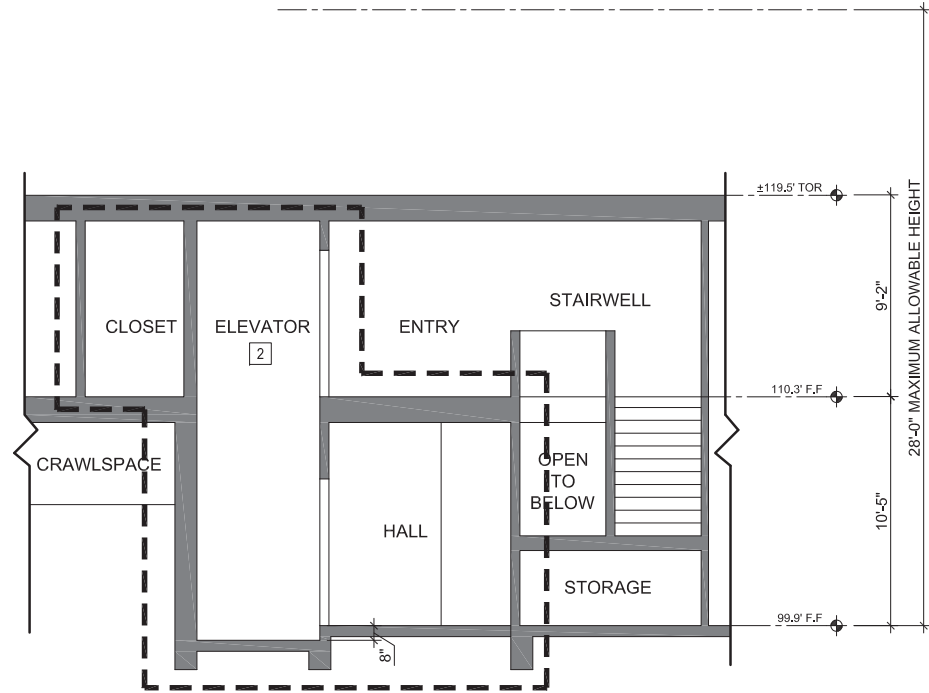
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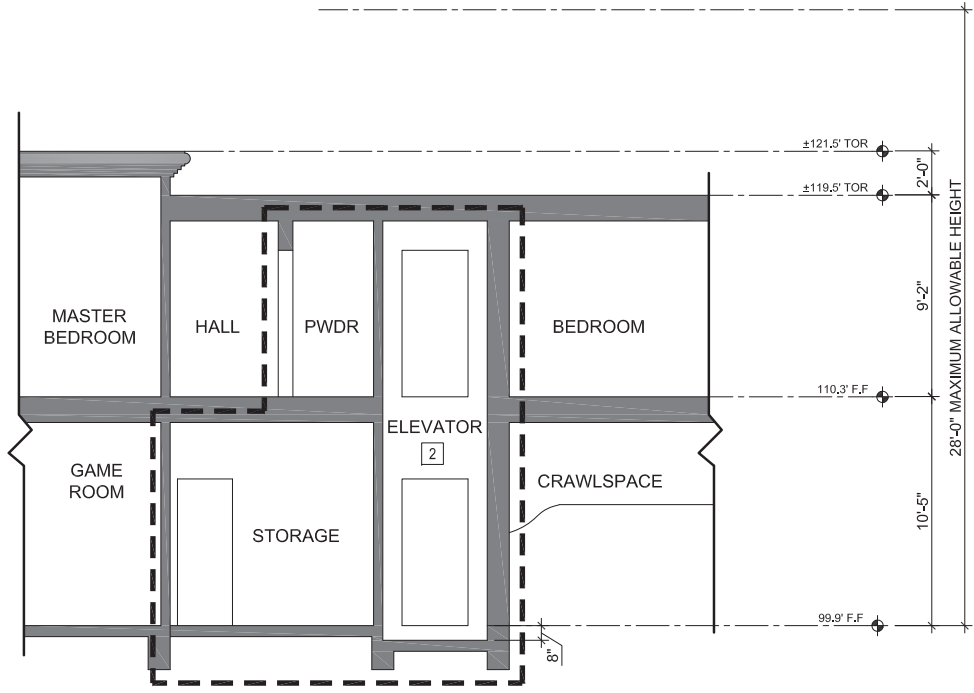
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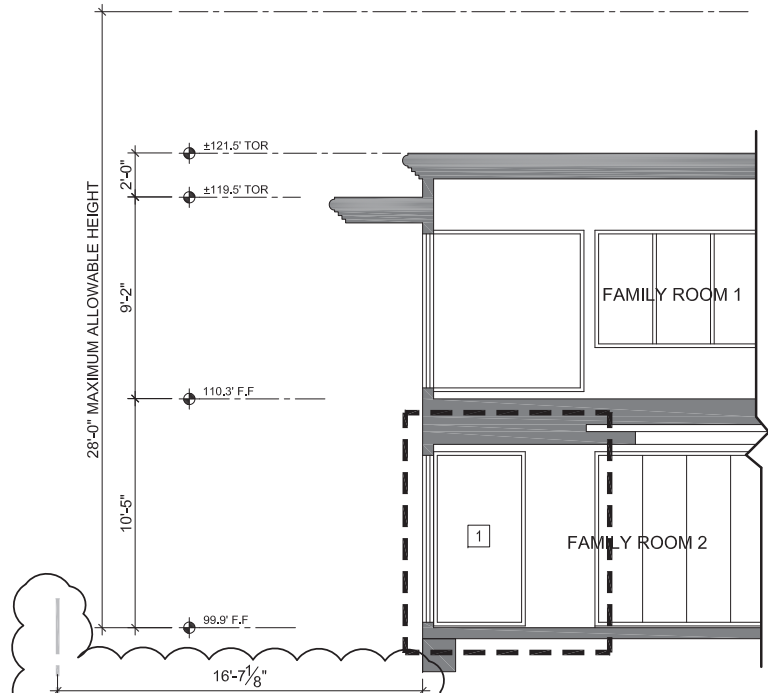
EXTERIOR ELEVATIONS
1/4"=1'-0"



PARTIAL BUILDING SECTION A-A
PROJECT AREA #1

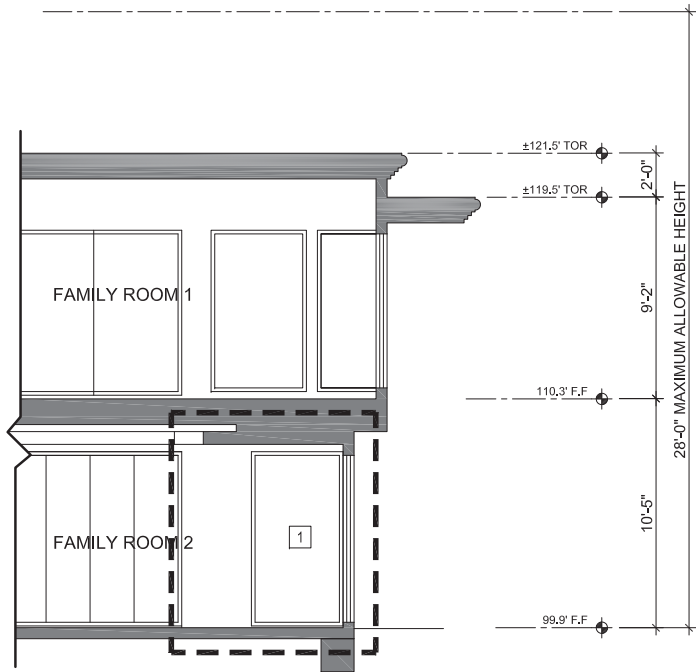


PARTIAL BUILDING SECTION B-B
PROJECT AREA #1



PARTIAL BUILDING SECTION C-C
PROJECT AREA #2

- KEY NOTES:**
1. INSTALL NEW 4'-6"x7'-6" (VERIFY SIZE) WINDOWS MATCHING THE EXISTING LAMINATED GLAZING, FRAME DESIGN, MATERIAL AND COLOR, AND INSTALLATION DETAILS.
 2. INSTALL GARAVENTA LIFT STYLE 1 IN-LINE DRIVE SYSTEM WITH A 40"x54" CAB SIZE AND A 3'-0"x6'-8" SOLID CORE SWINGING HALL DOOR AND SLIDING ELEVATOR CAR DOOR. VERIFY FINISHES WITH OWNER AND ARCHITECT.

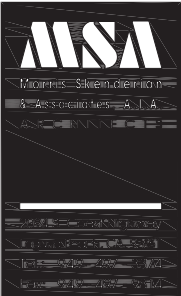


PARTIAL BUILDING SECTION D-D
PROJECT AREA #2

INDICATES
PROJECT
AREAS

PARTIAL BUILDING SECTIONS
1/4" = 1'-0"

REVISIONS		
NO.	DATE	BY
1	1/25/2019	DJB
2	2/20/2019	DJB

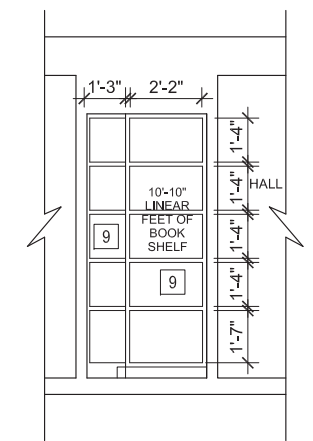
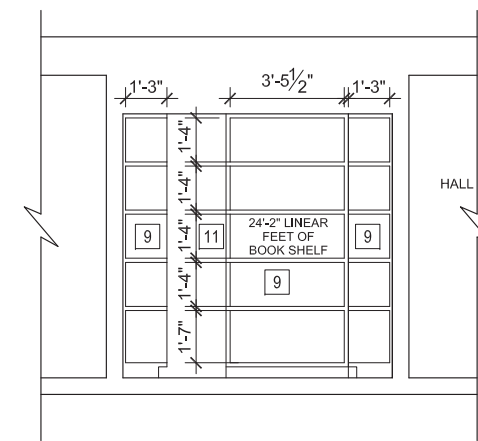
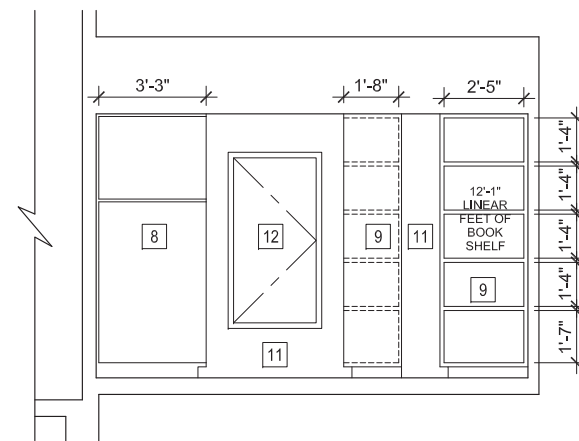
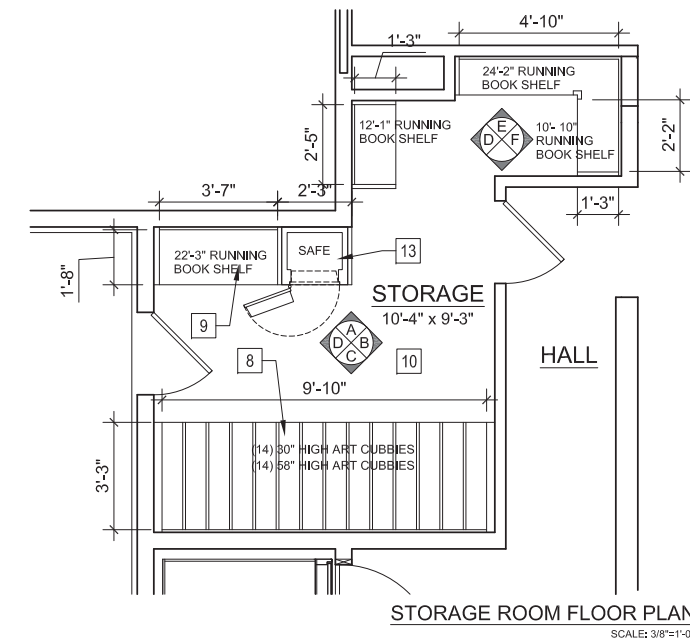
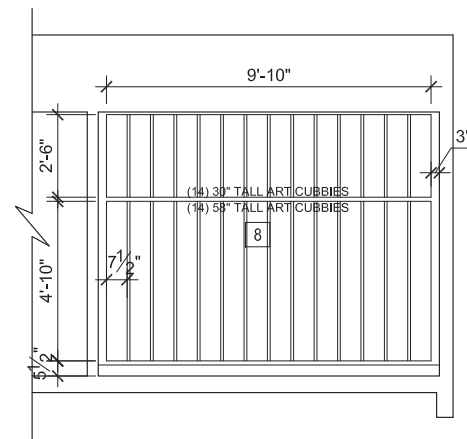
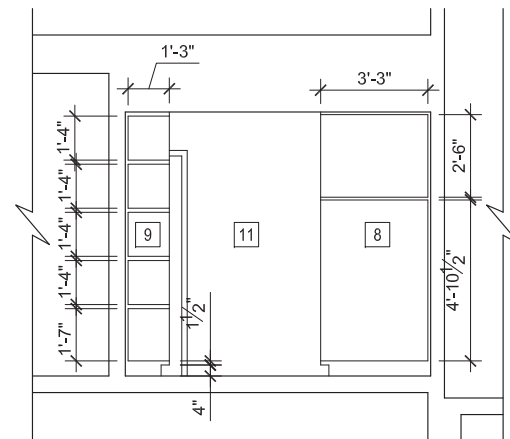
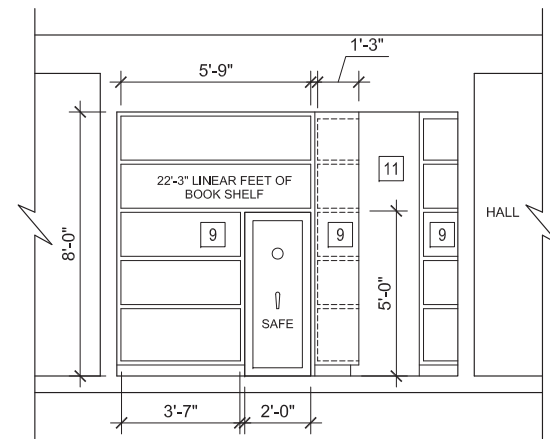


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PARTIAL BUILDING SECTIONS
STILES RESIDENCE REMODEL
87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

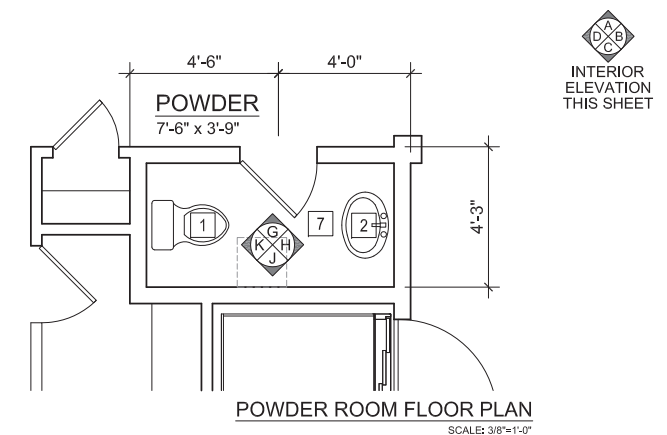
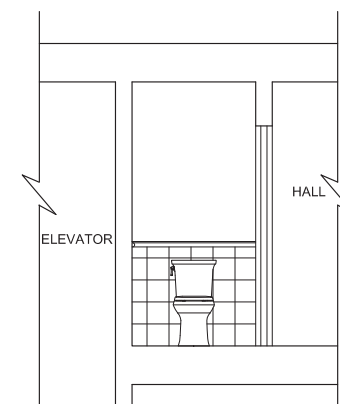
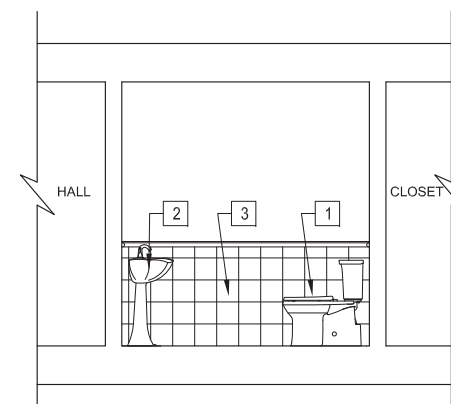
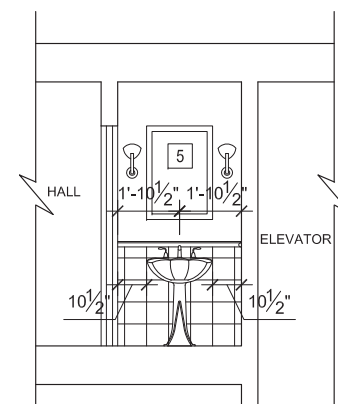
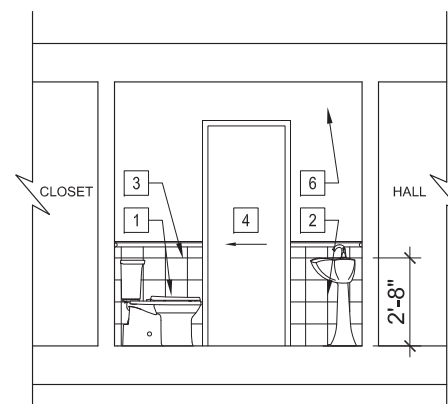
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- NOTE:

1. REINSTALL EXISTING TOILET IN NEW LOCATION.
2. REINSTALL EXISTING PEDESTAL SINK IN NEW LOCATION.
3. INSTALL CERAMIC WAINSCOT TO MATCH EXISTING.
4. INSTALL NEW SLIDING DOOR WITH CASING AND FINISHES TO MATCH ADJACENT SURFACES AND COLORS.
5. MIRROR AS SELECTED BY OWNER.
6. SMOOTH FINISH PAINTED DRYWALL FOR WET LOCATIONS - COLOR BY OWNER.
7. INSTALL TILE FLOORING TO MATCH EXISTING.

8. (14) 30" TALL & (14) 58" TALL ART STORAGE CUBBIES TO BE MELAMINE OR EQUAL WITH 1X2 EXTERIOR WOOD TRIM WITH EASED EDGES. BOTTOM OF EACH ART STORAGE CUBBY TO BE FITTED WITH REMOVABLE CARPET.
9. BOOK SHELVES TO BE MELAMINE OR EQUAL WITH 1X2 EXTERIOR WOOD TRIM WITH EASED EDGES. TOTAL OF 63'-9" OF LINEAR BOOK SHELVES PROVIDED.
10. INSTALL NEW FLOORING PER OWNER.
11. SMOOTH FINISH PAINTED DRYWALL - COLOR BY OWNER.
12. SOLID CORE CRAWL SPACE ACCESS DOOR - VERIFY SIZE.
13. NEW SAFE BY OWNER. CASORO OR EQUAL.



INTERIOR
ELEVATION
THIS SHEET

STORAGE AND POWDER INTERIOR ELEVATIONS

[illegible]

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STORAGE & POWDER ROOMS - INTERIOR ELEVATIONS.

STILES RESIDENCE REMODEL
87 MONARCH BAY DRIVE

57 MONARCH DAT DRIVE
DANA POINT, CALIFORNIA 92629

REVIEWED BY / DATE	
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MECHANICAL EXPANSION IN HARDENED CONCRETE

1. EXPANSION ANCHOR SYSTEMS:
- A. CONCRETE, SIMPSON STRONG-TIE STRONG-BOLT 2 (ICC-ESR-3037), USE ONLY EXPANSION ANCHOR SYSTEMS THAT HAVE BEEN PRE-QUALIFIED IN ACCORDANCE WITH THE PROVISIONS OF ICC-ES AC108, AND APPROVED FOR USE IN CRACKED CONCRETE. ANCHOR SYSTEMS SHALL BE INSTALLED PER THE REQUIREMENTS OF THE ICC-ES EVALUATION SERVICES REPORT FOR THE SPECIFIC ANCHOR, OR AS REQUIRED BY THE MANUFACTURER.
- B. CONCRETE, SIMPSON TITEN HD (ICC-ESR-2713), AND APPROVED FOR ONLY EXPANSION ANCHOR SYSTEMS THAT HAVE BEEN PRE-QUALIFIED IN ACCORDANCE WITH THE PROVISIONS OF ICC-ES AC108, AND APPROVED FOR USE IN CRACKED CONCRETE. ANCHOR SYSTEMS SHALL BE INSTALLED PER THE REQUIREMENTS OF THE ICC-ES EVALUATION SERVICES REPORT FOR THE SPECIFIC ANCHOR, OR AS REQUIRED BY THE MANUFACTURER.
- C. ANCHOR DIAMETER REFERS TO THE ANCHOR SIZE AND NOT THE DIAMETER OF THE HOLE. REFER TO THE MANUFACTURER'S INSTRUCTIONS FOR REQUIRED DIAMETER OF DRILLED HOLE.
2. WHERE THE MANUFACTURER'S INSTALLATION INSTRUCTIONS OR APPLICABLE CODES CALL FOR THE APPLICATION OF AN INSTALLATION TORQUE, THE SPECIFIED TORQUE SHALL BE APPLIED WITH A CALIBRATED TORQUE WRENCH. FOLLOWING ATTAINMENT OF 10% OF THE SPECIFIED TORQUE, 100% OF THE SPECIFIED TORQUE SHALL BE REACHED WITHIN 7 OR FEWER COMPLETE TURNS OF NUT. THE SPECIFIED INSTALLATION TORQUE SHALL NOT BE EXCEEDED.
3. USE OF ZINC-COATED CARBON STEEL ANCHORS IS LIMITED TO DRY, INTERIOR LOCATIONS, UNLESS OTHERWISE NOTED. PROVIDE STAINLESS STEEL OR HOT-DIP GALVANIZED ANCHORS FOR APPLICATIONS EXPOSED TO EXTERIOR WEATHER CONDITIONS.
4. EXPANSION ANCHORS FOR NON-VIBRATION ISOLATED MECHANICAL EQUIPMENT RATED OVER 10 HP ARE NOT PERMITTED BY ASCE 7-05 SECTION 13.6.3. EXPANSION ANCHORS INSTALLED IN OVERHEAD CONDITIONS FOR NON-VIBRATION ISOLATED EQUIPMENT WITH RECIPROCATING OR ROTATING MECHANISMS SHALL BE UNDERCUT ANCHORS.
5. WHERE MECHANICAL ANCHORS ARE USED IN A STANDOFF CONFIGURATION (I.E., WHERE THE ATTACHMENT IS PREPARED FROM THE CONCRETE IN WHICH THE ANCHOR IS INSTALLED), A NUT AND WASHER SHALL BE PROVIDED AT THE CONCRETE SURFACE TO FACILITATE SETTING OF THE ANCHOR AND TO TRANSMIT AXIAL COMPRESSION LOADS INTO CONCRETE.
6. THE SPECIAL INSPECTOR SHALL BE ON THE JOBSITE CONTINUOUSLY DURING ANCHOR INSTALLATIONS, UNLESS OTHERWISE NOTED IN ICC-ES ESR, TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE COMPRESSIVE STRENGTH, HOLE DIMENSIONS, ANCHOR SPACINGS, EDGE DISTANCES, SLAB THICKNESS, ANCHOR EMBEDMENT, AND TIGHTENING TORQUE.
7. THE TENSION TESTING OF THE MECHANICAL ANCHORS SHALL BE DONE IN THE PRESENCE OF THE SPECIAL INSPECTOR AND REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO THE ENFORCEMENT AGENCY. IF ANY ANCHORS FAIL THE TENSION-TESTING REQUIREMENTS, THE ADDITIONAL TESTING REQUIREMENTS SHALL BE ACCEPTABLE TO THE ENFORCEMENT AGENCY.
8. TEST QUANTITY OF ANCHORS AS NOTED BELOW:
- | APPLICATION | QUANTITY |
|----------------|-------------------------------------|
| STRUCTURAL | 100% OF BOLTS (10% FOR SILL PLATES) |
| NON-STRUCTURAL | 50% OF BOLTS |
9. ANCHORS TO BE TESTED SHALL BE SELECTED AT RANDOM BY THE INSPECTOR.
10. ALL TESTING SHALL BE PERFORMED A MINIMUM OF 24 HOURS AFTER INSTALLATION.
11. UNDERCUT ANCHORS THAT ALLOW VISUAL CONFIRMATION OF FULL SET NEED NOT TO BE TESTED, UNLESS OTHERWISE NOTED BY ENFORCEMENT AGENCY OR ENGINEER OF RECORD.
12. WHERE THE DESIGN TENSION ON ANCHORS IS LESS THAN 100 POUNDS AND THOSE ANCHORS ARE CLEARLY IDENTIFIED ON THE CONTRACT DOCUMENTS, ONLY 10 PERCENT OF THOSE ANCHORS NEED BE TESTED, UNLESS OTHERWISE NOTED BY ENFORCEMENT AGENCY OR ENGINEER OF RECORD.
13. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:
- A. HYDRAULIC RAM METHOD: THE ANCHOR SHALL HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR NON-HANGAR ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENTS IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.
- B. TORQUE WRENCH METHOD: THE APPLICABLE TEST TORQUE MUST BE REACHED WITHIN ONE-HALF (1/2) TURN OF THE NUT.
- C. USE OF CALIBRATED SPRING LOADED DEVICES.
14. IF ANY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME TYPE, INSTALLED BY THE SAME TRADE, NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE ANCHORS PASS, THEN RESUME THE INITIAL TEST FREQUENCY.
15. WHEN INSTALLING DRILLED-IN ANCHORS IN EXISTING NON-PRESTRESSED REINFORCED CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A MINIMUM CLEARANCE OF ONE INCH BETWEEN THE REINFORCEMENT AND DRILLED-IN ANCHOR.
16. IF REBAR IS ENCOUNTERED DURING THE DRILLING, THE CONTRACTOR SHALL IMMEDIATELY TERMINATE DRILLING AND NOTIFY THE ENGINEER OF RECORD. THE ENGINEER OF RECORD WILL AUTHORIZE USING ONE OF THE FOLLOWING PROCEDURES:
- A. IF THE ANCHOR MAY BE SHIFTED, FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. THE MINIMUM SPACING BETWEEN AN ABANDONED HOLE AND A DRILLED HOLE USED FOR A POST-INSTALLED ANCHOR SHALL NOT BE LESS THAN 1-1/2 ANCHOR DIAMETERS UNLESS OTHERWISE APPROVED BY THE ENFORCEMENT AGENCY.
- B. IF THE ANCHOR LOCATION MAY NOT BE SHIFTED, CORE AN OVERSIZE HOLE AT THE DIRECTION OF THE ENGINEER OF RECORD AND GROUT AN APPROVED ANCHOR PLACE.
17. IF THE CONCRETE CRACKS DURING THE INSTALLATION OF THE ANCHOR, THE ANCHOR SHALL BE REMOVED OR ABANDONED.
18. FOR THE ANCHORS INSTALLED INTO CONCRETE USING SIMPSON STRONG-BOLT 2 (ICC-ESR 3037, LARR 25991),

STRUCTURAL CONCRETE	
ANCHOR DIAMETER	TORQUE (ft-lb)
3/8"	30
1/2"	60
5/8"	80
3/4"	150

CONCRETE OVER METAL DECK		
ANCHOR DIAMETER	PLACED IN LOWER FLUTE	PLACED IN UPPER FLUTE
TORQUE (ft-lb)		
3/8"	30	30
1/2"	60	60
5/8"	80	-
3/4"	150	-

TITEN HD SCREW ANCHOR		TITEN HD ROD HANGER	
ANCHOR DIAMETER	TORQUE (ft-lb)	ANCHOR DIAMETER	TORQUE (ft-lb)
3/8"	30	3/8"	50
1/2"	65	1/2"	50
3/4"	150		

19. FOR THE ANCHORS INSTALLED INTO CONCRETE USING SIMPSON TITEN HD (ICC-ESR 2713, LARR 25914), THE TESTING LABORATORY WILL USE THE FOLLOWING TORQUE VALUES:

TITEN HD SCREW ANCHOR		TITEN HD ROD HANGER	
ANCHOR DIAMETER	TORQUE (ft-lb)	ANCHOR DIAMETER	TORQUE (ft-lb)
3/8"	30	3/8"	50
1/2"	65	1/2"	50
3/4"	150		

WOOD

1. ALL STRUCTURAL LUMBER SHALL BE DOUGLAS FIR-LARCH, VISUALLY GRADED OR MACHINE GRADED UNDER THE STANDARD GRADING RULES, NO. 17, OF THE WEST COAST LUMBER INSPECTION BUREAU, AND SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 23 OF THE CBC. ALL FRAMING SHALL BE LESS THAN 19% MOISTURE CONTENT AT TIME OF INSTALLATION. ALL FRAMING MEMBERS SHALL BE AS FOLLOWS:
2. WALL FRAMING TO BE 2X STUDS AT 16 INCHES O.C. ON EXTERIOR AND INTERIOR WALLS. UNO. PROVIDE DOUBLE TOP PLATE ON ALL WALLS WITH MINIMUM 4" LAP SPLICE. UNLESS SPECIFICALLY NOTED ON PLANS, MAXIMUM STUD HEIGHT FOR 2x4 INTERIOR WALLS IS 14'-0" AND MAXIMUM STUD HEIGHT FOR 2x6 NON-BEARING INTERIOR WALLS IS 20'-0".
- | SIZE AND APPLICATION | GRADE |
|--|-------------------|
| 2x TOP PLATES & SILLS | STANDARD OR NO. 2 |
| 2x4 STUDS (UP TO 9'-0") | STUD |
| 2x6 STUDS (OVER 9'-0") | NO. 2, UNO |
| 2x6 STUDS (UP TO 10'-0") | STUD |
| 2x6 STUDS (OVER 10'-0") | NO. 2, UNO |
| 6x AND LARGER BEAMS AND POST | NO. 1, UNO |
| JOIST AND ALL OTHER 2" TO 4" SAWN LUMBER | NO. 2, UNO |
3. ALL WOOD BEARING ON CONCRETE OR MASONRY IN DIRECT CONTACT WITH EARTH SHALL BE PRESERVE TREATED DOUGLAS FIR IN ACCORDANCE WITH CBC 2303.19 AND 2304.12. ALL CUTS, HOLES & NOTCHES BE TREATED WITH A PRESERVATIVE APPROVED BY THE STRUCTURAL ENGINEER.
4. FASTENERS FOR PRESERVE-PRESERVATIVE TREATED WOOD SHALL BE HOT-DIPPED ZINC COATED GALVANIZED, STAINLESS STEEL, SILICON BRONZE OR COPPER PER CBC 2304.10.5. EXCEPTION: PLAIN CARBON STEEL FASTENERS INCLUDING NUTS AND WASHERS IN SBX-DOT AND ZINC BORATE PRESERVATIVE-TREATED WOOD IN AN INTERIOR, DRY ENVIRONMENT SHALL BE PERMITTED.)
5. ALL STRUCTURAL SHEATHING SHALL BE DOUGLAS FIR STANDARD GRADE STRUCTURAL WITH EXTERIOR GLUE conforming TO THE LATEST EDITIONS OF PS-1 AND PS-2 UNO. ALL PANELS SHALL BEAR LEGIBLE APA STAMPS. NO INDIVIDUAL SHEETS SHALL BE LESS THAN 2 FEET IN WIDTH. FULL SHEETS (4'-0"x8'-0") SHALL BE USED WHEREVER POSSIBLE.
6. ALL FLOOR AND ROOF SHEATHING SHALL BE LAD FACE GRAIN PERPENDICULAR TO FRAMING UNO. ALL SHEATHING SHALL BE APPROVED BY THE BUILDING INSPECTOR BEFORE COVERING.
7. UNLESS NOTED OTHERWISE, ALL WOOD SILL PLATE UNDER BEARING, EXTERIOR, OR SHEAR WALLS IN CONTACT WITH CONCRETE OR MASONRY SHALL BE BOLTED TO CONCRETE OR MASONRY WITH 1/2" Ø ANCHOR RODS w/ 7" MIN CONCRETE / MASONRY EMBED AT 4'-0" OC & AT 4" MIN AND 12" MAX FROM EACH END OF THE PLATES. A MINIMUM OF TWO ANCHOR BOLTS SHALL BE PLACED IN ANY ONE PIECE OF SILL PLATE. THESE AND ALL ADDITIONAL REQUIREMENTS OF CBC 2304.12.2 & 2304.12.3 SHALL BE FOLLOWED IN THE INSTALLATION OF WALL SILL PLATES.
8. ALL BOLT HEADS AND NUTS WHICH BEAR AGAINST THE FACE OF WOOD MEMBERS SHALL BE PROVIDED WITH 2x2x3/16" STEEL PLATE WASHERS UNO. ALL ANCHOR BOLTS FOR SHEAR WALLS SHALL BE PROVIDED WITH A 3"x3"x22" MIN STEEL PLATE WASHER UNO ON SHEAR WALL SCHEDULE.
9. ALL NAILS FOR CONNECTING WOOD MEMBERS SHALL BE COMMON NAILS w/ A MECHANICAL PULL-OUT RESISTANCE. MINIMUM NAILING SCHEDULES ARE OUTLINED IN TABLE 2304.10.1 SHALL BE FOLLOWED UNLESS NOTED OTHERWISE.
10. RETIGHTEN BOLTS BEFORE CLOSING-IN.
11. USE OF MACHINE NAILING IS SUBJECT TO A SATISFACTORY JOBSITE DEMONSTRATION FOR EACH PROJECT AND THE APPROVAL BY THE ARCHITECT OR STRUCTURAL ENGINEER AND THE INSPECTOR OF RECORD. THE APPROVAL IS SUBJECT TO CONTINUED SATISFACTORY PERFORMANCE. MACHINE NAILING WILL NOT BE APPROVED IN 8" SP SHEATHING. IF NAIL HEADS PENETRATE THE OUTER PLY MORE THAN WOULD BE NORMAL FOR A HAND HAMMER OR IF MINIMUM ALLOWABLE EDGE DISTANCES ARE NOT MAINTAINED, THE PERFORMANCE WILL BE DEEMED UNSATISFACTORY.
12. ALL WOOD HARDWARE CONNECTORS SHALL BE SIMPSON STRONG-TIE UNLESS NOTED OTHERWISE.
13. PROVIDE FULL DEPTH BLOCKING AT 4'-0" OC BETWEEN ADJACENT JOISTS UNDER ALL PERPENDICULAR OR PARALLEL JOIST UNDER ALL PARALLEL PARTITIONS UNLESS NOTED OTHERWISE.
14. UNLESS SPECIFICALLY SHOWN ON THESE PLANS, NO STRUCTURAL MEMBER SHALL BE CUT, DRILLED, NOR NOTCHED WITHOUT PRIOR WRITTEN AUTHORIZATION FROM THE NOTICE.
15. ROOF SHEATHING SHALL BE INSPECTED PRIOR TO PLACING INSULATION AND ROOFING.
16. SOLID BLOCKING (2x MIN) SHALL BE PLACED BETWEEN JOISTS OR RAFTERS AT ALL SUPPORTS & @ 8'-0" OC MAX.
17. ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554 GR 36. ALL BOLTS SHALL CONFORM TO ASTM A307 UNO.
18. HOLES FOR BOLTS SHALL BE BORED 1/32" TO 1/16" LARGER THAN NOMINAL BOLT DIAMETER. (THIS INCLUDES HOLES FOR ANCHOR RODS IN SILL PLATE).
19. JOIST CONNECTIONS SHALL BE SIMPSON LUS UNLESS NOTED OTHERWISE.
20. ALL POSTS CONNECTION AT TOP AND BOTTOM SHALL BE SIMPSON VC2 UNO.
21. ALL POST CONNECTIONS LOCATED DIRECTLY ON CONCRETE SHALL BE SIMPSON TCB UNLESS NOTED OTHERWISE.
22. FILL ALL NAIL HOLES OF WOOD CONNECTION HARDWARE (STRAPS, HANGERS, ETC.) WITH PUTTY OR EPOXY.
23. ALL NON-SHEAR WALLS SHALL HAVE BRACING PROVIDED BY ONE OF THE METHODS REQUIRED BY CODE SECTION 2308.6.
24. ALL LAG SCREWS AND WOOD SCREWS SHALL CONFORM TO ANSI/AIAE STANDARD B19.2.1 WITH CUT HEADS.
25. SECURE ALL EMBEDDED HARDWARE PRIOR TO FOUNDATION INSPECTION.
26. LAG SCREWS, ANSI/AIAE STANDARD CUP B19.2.1-2015 NDS FOR LAG SCREW DIMENSIONS. PRE-DRILL ALL HOLES. HOLE AT SHANK PORTION TO MATCH DIAMETER OF SHANK. HOLES AT THREADED PORTION TO BE 60 TO 70 PERCENT OF SHANK DIAMETER AND EQUAL TO LENGTH OF THREADED PORTION UNO. USE SOAP AND LUBRICANTS TO FACILITATE INSTALLATION. DRIVING WITH HAMMER IS NOT PERMITTED. EXPOSED STEEL SHALL BE HOT-DIPPED GALVANIZED.

27. LATERAL SUPPORT FOR BEAMS, RAFTERS AND JOISTS: PER CBC 2308.4.2.3 (JOISTS SHALL BE SUPPORTED LATERALLY AT THE ENDS AND AT EACH SUPPORT BY SOLID BLOCKING EXCEPT WHERE THE ENDS OF JOISTS ARE NAILED TO A HEADER, AND/OR RM JOIST OR TO AN ADJOINING STUD OR BY OTHER APPROVED MEANS. SOLID BLOCKING SHALL NOT BE LESS THAN 2 INCHES (51 mm) IN THICKNESS AND THE FULL DEPTH OF JOIST.)
28. BEAMS OR DRAG STRUTS, LAMINATED OR DOUBLE JOIST SHALL BE SPIRED TOGETHER WITH 16d NAILS AT 9" OC STAGGERED. IF USING 3 OR MORE JOIST TOGETHER, CONNECT WITH 1/2" DIA MACHINE BOLTS @ 24" OC STAGGERED.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED BY AN APPROVED AND LICENSED FABRICATOR IN ACCORDANCE WITH THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (LATEST EDITION), AND WITH CHAPTER 22A OF THE CODE.
2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE ASTM DESIGNATION AS INDICATED BELOW (UNO):
- | W/F AND W/T SHAPES | A500 |
|---------------------------|---------------------|
| PIPE COLUMNS | A513, GRADE B |
| HSS SECTIONS | A500, GRADE B |
| HIGHT-STRENGTH BOLTS | A325 |
| BOLTS IN CONCRETE/MASONRY | A307 |
| ANGLES, CHANNELS, PLATES | A36 |
| ANCHOR RODS | F1554, GRADE 36 UNO |
3. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS TO THE ARCHITECT OF ALL STEEL FOR ARCHITECTS AND STRUCTURAL ENGINEERS REVIEW AND APPROVAL BEFORE FABRICATION.
4. BOLT HOLES USED IN STEEL SHALL BE 1/16" LARGER IN DIAMETER THAN NOMINAL SIZE OF BOLT USED, EXCEPT AS NOTED.
5. ALL STRUCTURAL STEEL SURFACES THAT ARE ENCASED IN CONCRETE, OR MASONRY, SPRAY ON FIREPROOFING, OR ARE ENCASED BY BUILDING FINISH, SHALL BE LEFT UNPAINTED.
6. ALL WELDING IS TO BE DONE BY CERTIFIED WELDERS APPROVED USING E70XX ELECTRODES (UNO). ALL WELDS SHALL BE IN CONFORMITY WITH THE PROJECT SPECIFICATIONS AND THE CODE FOR WELDING IN BUILDING CONSTRUCTION (AWS D11.1 LATEST REVISION) OF THE AMERICAN WELDING SOCIETY. SEE SPECIAL INSPECTIONS SECTION FOR WELDING INSPECTION REQUIREMENTS.
7. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL CONSTRUCTION 13TH EDITION, TABLE J2.4.
8. ALL EXPOSED STRUCTURAL STEEL AND MISCELLANEOUS METAL SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.
9. ALL GROOVE WELDS SHALL BE FULL PENETRATION UNLESS NOTED OTHERWISE.
10. WHERE FINISH IS ATTACHED TO STRUCTURAL STEEL PROVIDE 1/2"DIA. BOLT HOLES @ 48" OC @ W, C, L MEMBERS AND 1/2" DIA. WELDED STUDS @ 48" OC @ TUBE & PIPE MEMBERS FOR ATTACHMENT OF NAILERS, SEE ARCHITECTURAL DRAWINGS FOR FINISHES.
11. SEISMIC LOAD RESISTING SYSTEM (SRS) IS DEFINED AS THE ASSEMBLY OF STRUCTURAL ELEMENTS IN THE BUILDING THAT RESISTS SEISMIC LOADS, INCLUDING AND INCLUDING: COLUMNS, BEAMS, CORNERS, CHORDS AND BRACES, AND THE CONNECTIONS BETWEEN THESE ELEMENTS SPECIFICALLY DESIGN TO RESIST SEISMIC FORCES, AS DESIGNATED ON THE DRAWINGS.
12. WELD MATERIALS USED IN SRS WELD CONNECTIONS SHALL CONFORM TO THE FOLLOWING MINIMUM STRENGTH REQUIREMENTS OF CORAS:
- A. WELD CONNECTIONS SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT 0 DEGREES F AS DETERMINED BY THE APPROPRIATE AWS CLASSIFICATION TEST METHOD.
- B. WELDED CONNECTIONS DESIGNATED AS "DEMAND CRITICAL" SHALL BE MADE WITH A FILLER METAL CAPABLE OF PROVIDING A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT 20 DEGREES F AS DETERMINED BY THE APPROPRIATE AWS CLASSIFICATION TEST METHOD, AND 40 FT-LB AT 70 DEGREES F AS DETERMINED BY APPENDIX X OF ANS/AISC 341.

13. THE ACCEPTANCE CRITERIA FOR INSTALLED ANCHORS IS AS FOLLOWS:
- HYDRAULIC RAM METHOD: THE ANCHOR SHOULD HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.
- TORQUE WRENCH METHOD: THE APPLICABLE TEST TORQUE MUST BE REACHED WITHIN THE FOLLOWING LIMITS:
- FOR WEDGE OR SLEEVE-TYPE ANCHORS: ONE HALF (1/2) TURN OF THE NUT. FOR THE 3/8 IN SLEEVE ANCHOR ONLY: ONE QUARTER (1/4) TURN OF THE NUT.

MECHANICAL EXPANSION ANCHORS IN CMU

1. ANCHOR DIAMETER REFERS TO THE ANCHOR SIZE AND NOT THE DIAMETER OF THE DRILLED HOLE. FOLLOW MANUFACTURER'S INSTRUCTIONS FOR DIAMETER OF DRILLED HOLE.
2. ALL ANCHORS SHOULD MEET THE MINIMUM EMBEDMENT, EDGE DISTANCE, SPACING AND SLAB THICKNESS CRITERIA ESTABLISHED BY THE RELEVANT ICC-ES EVALUATION REPORT.
3. REFER TO MPM ER-240 FOR SIMPSON STRONG BOLT 2. REFER TO ICC-ESR 1096 FOR SIMPSON STRONG-BOLT ANCHOR TITEN HD.
4. WHEN INSTALLING DRILLED-IN ANCHORS AND/OR PULDOWN DRIVEN PINS IN EXISTING REINFORCED MASONRY, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS.
5. IT IS NOT ACCEPTABLE TO SUBSTITUTE ANY CAST-INPLACE BOLTS/RODS/ANCHORS FOR EXPANSION ANCHORS WITHOUT PRIOR APPROVAL FROM THE ENGINEER OF RECORD.
6. USE HOT-DIP GALVANIZED OR STAINLESS STEEL ANCHORS WHEN EXPANSION ANCHORS ARE EXPOSED TO WEATHER OR IN A DAMP ENVIRONMENT.
7. DESIGN LOADS ARE SHOWN IN CC REPORTS DESCRIBED ABOVE. THEY CANNOT BE INCREASED BY 1.33 FOR SEISMIC OR WIND FORCES.
8. WHEN EXPANSION-TYPE ANCHORS ARE USED FOR SILL PLATE BOLTING APPLICATIONS, 10 PERCENT OF THE ANCHORS SHALL BE TESTED.
9. WHEN EXPANSION-TYPE ANCHORS ARE USED FOR OTHER STRUCTURAL APPLICATIONS, ALL SUCH EXPANSION ANCHORS SHALL BE TESTED.
10. WHEN EXPANSION-TYPE ANCHORS ARE USED FOR NON-STRUCTURAL APPLICATIONS SUCH AS EQUIPMENT ANCHORAGE, 50 PERCENT OR ALTERNATE BOLTS IN A GROUP, INCLUDING AT LEAST HALF THE ANCHORS IN EACH GROUP, SHALL BE TESTED.
11. THE TESTING OF THE EXPANSION BOLTS SHALL BE DONE IN THE PRESENCE OF THE PROJECT INSPECTOR AND REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO THE ENFORCEMENT AGENCY.
12. IF ANY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME CATEGORY, NOT PREVIOUSLY TESTED, UNTIL TWENTY (20) CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENTS. THE INITIAL TEST FREQUENCY SHALL BE INCREASED BY 1.33 FOR SEISMIC OR WIND FORCES.
13. THE ACCEPTANCE CRITERIA FOR INSTALLED ANCHORS IS AS FOLLOWS:
- HYDRAULIC RAM METHOD: THE ANCHOR SHOULD HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.

NOMINAL BAR SIZE	EMBEDMENT DEPTH	NORMAL - WEIGHT CONCRETE
DIA (IN)	H (IN)	F _{cr} = 3000 PSI
#4	2.34	3456
#5	2.6	7540
#6	3	7554
#5	5	1751
#6	9	16965

14. THE TESTING OF THE EXPANSION BOLTS SHALL BE DONE IN THE PRESENCE OF THE PROJECT INSPECTOR AND REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO THE ENFORCEMENT AGENCY.
15. IF ANY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME CATEGORY, NOT PREVIOUSLY TESTED, UNTIL TWENTY (20) CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENTS. THE INITIAL TEST FREQUENCY SHALL BE INCREASED BY 1.33 FOR SEISMIC OR WIND FORCES.
16. THE ACCEPTANCE CRITERIA FOR INSTALLED ANCHORS IS AS FOLLOWS:
- HYDRAULIC RAM METHOD: THE ANCHOR SHOULD HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.

17. LATERAL SUPPORT FOR BEAMS, RAFTERS AND JOISTS: PER CBC 2308.4.2.3 (JOISTS SHALL BE SUPPORTED LATERALLY AT THE ENDS AND AT EACH SUPPORT BY SOLID BLOCKING EXCEPT WHERE THE ENDS OF JOISTS ARE NAILED TO A HEADER, AND/OR RM JOIST OR TO AN ADJOINING STUD OR BY OTHER APPROVED MEANS. SOLID BLOCKING SHALL NOT BE LESS THAN 2 INCHES (51 mm) IN THICKNESS AND THE FULL DEPTH OF JOIST.)
28. BEAMS OR DRAG STRUTS, LAMINATED OR DOUBLE JOIST SHALL BE SPIRED TOGETHER WITH 16d NAILS AT 9" OC STAGGERED. IF USING 3 OR MORE JOIST TOGETHER, CONNECT WITH 1/2" DIA MACHINE BOLTS @ 24" OC STAGGERED.

EPOXY ANCHORS AND REBAR IN HARDENED CONCRETE

1. EPOXY ANCHOR SYSTEMS:
- A. CONCRETE, SIMPSON SET-UP (ICC-ESR-2506) USE ONLY ADHESIVE ANCHOR SYSTEMS THAT HAVE BEEN PRE-QUALIFIED IN ACCORDANCE WITH THE PROVISIONS OF ICC-ES AC308, APPROVED FOR USE IN CRACKED CONCRETE. ANCHOR SYSTEMS SHALL BE INSTALLED PER THE REQUIREMENTS OF THE ICC-ES EVALUATION SERVICES REPORT FOR THE SPECIFIC ANCHOR, OR AS REQUIRED BY THE MANUFACTURER.
2. REINFORCEMENT BARS SHALL BE ASTM A615 GRADE 60 STEEL, THREADED BARS SHALL BE ASTM F1554 OR 36 (PLAIN) OR ASTM A193 GR B8 (STAINLESS).
3. REMOVE GREASE OIL, RUST AND ANY OTHER LAITANCE FROM RODS AND DOMELS PRIOR TO INSTALLATION.
4. THE SPECIAL INSPECTOR SHALL BE ON THE JOBSITE CONTINUOUSLY DURING ANCHOR INSTALLATIONS, UNLESS OTHERWISE NOTED IN ICC-ES ESR, TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE COMPRESSIVE STRENGTH, HOLE DIMENSIONS, ANCHOR SPACING, EDGE DISTANCES, SLAB THICKNESS, ANCHOR EMBEDMENT, AND TIGHTENING TORQUE.
5. THE TENSION TESTING OF THE CHEMICAL ANCHORS SHALL BE DONE IN THE PRESENCE OF THE SPECIAL INSPECTOR AND REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO THE ENFORCEMENT AGENCY. IF ANY ANCHORS FAIL THE TENSION-TESTING REQUIREMENTS, THE ADDITIONAL TESTING REQUIREMENTS SHALL BE ACCEPTABLE TO THE ENFORCEMENT AGENCY.
6. TEST QUANTITY OF ANCHORS AS NOTED BELOW:
- | APPLICATION | QUANTITY |
|----------------|---------------|
| STRUCTURAL | 100% OF BOLTS |
| NON-STRUCTURAL | 50% OF BOLTS |
7. ANCHORS TO BE TESTED SHALL BE SELECTED AT RANDOM BY THE SPECIAL INSPECTOR.
8. ALL TESTING SHALL BE PERFORMED A MINIMUM OF 24 HOURS AFTER INSTALLATION OF THE SUBJECT ANCHORS.
9. WHERE ADHESIVE ANCHOR SYSTEMS ARE USED TO INSTALL REINFORCING DOVEL BARS IN HARDENED CONCRETE, ONLY 25% OF THE DOVELS NEED BE TESTED IF THE FOLLOWING CONDITIONS ARE MET:
- A. THE DOVELS ARE USED EXCLUSIVELY TO TRANSMIT SHEAR FORCES ACROSS JOINTS BETWEEN EXISTING AND NEW CONCRETE.
- B. THE NUMBER OF DOVELS IN ANY ONE MEMBER EQUALS OR EXCEEDS 12.
- C. THE DOVELS ARE UNIFORMLY DISTRIBUTED ACROSS SEISMIC FORCE RESISTING MEMBERS (SUCH AS SHEAR WALLS, COLLECTORS AND DIAPHRAGMS).
10. TESTING OF SHEAR DOVELS AGAINST COLD JOINTS IN SLAB ON GRADE WHERE THE SLAB IS NOT PART OF THE LATERAL FORCE-RESISTING SYSTEM IS NOT REQUIRED.
11. REPLACE ANCHORS AND DOVELS THAT FAIL DURING TESTING AND RETEST. IF MORE THAN 10% OF THE TESTED ANCHORS AND ANCHOR DOVELS FAIL TO MEET THE SPECIFIED TEST LOAD, TEST 100% OF THE DOVELS AND ANCHORS IN THE LAST 2 DAYS OF ANCHOR INSTALLATION.
12. A HYDRAULIC CYLINDER SHALL BE USED TO APPLY THE TENSION TEST LOAD TO THE ANCHOR WITH THE CYLINDER SUPPORTED ON A LOADING PLATE. THE DRILLED HOLE MAY NOT BE REUSED. ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT. THE MINIMUM SPACING BETWEEN AND ABANDONED HOLE AND A DRILLED HOLE USED FOR A POST-INSTALLED ANCHOR SHALL NOT BE LESS THAN 1 1/2 ANCHOR DIAMETERS UNLESS OTHERWISE APPROVED BY THE ENFORCEMENT AGENCY. IF THE ANCHOR OR DOVEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER OR RECORD WILL DETERMINE A NEW LOCATION.
13. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:
- A. HYDRAULIC RAM METHOD: THE ANCHOR SHALL HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD.
14. IF ANY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME TYPE, INSTALLED BY THE SAME TRADE, NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE ANCHORS PASS, THEN RESUME THE INITIAL TEST FREQUENCY.
15. ALL HOLES FOR POST-INSTALLED ANCHORS SHALL BE DRILLED, CLEANED AND PREPARED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS OR THE APPLICABLE ICC-ESR. ALL DEBRIS SHALL BE REMOVED BY IN-HOLE BRUSHING COMBINED WITH VACUUM OR OIL-FREE COMPRESSED AIR. JETTING HOLES WITH WATER IS NOT PERMITTED. WHERE AN ANCHOR DOES NOT SET PROPERLY, OR FAILS A TENSION TEST, OR REINFORCEMENT IS ENCOUNTERED DURING DRILLING, THE DRILLED HOLE MAY NOT BE REUSED. ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT. THE MINIMUM SPACING BETWEEN AND ABANDONED HOLE AND A DRILLED HOLE USED FOR A POST-INSTALLED ANCHOR SHALL NOT BE LESS THAN 1 1/2 ANCHOR DIAMETERS UNLESS OTHERWISE APPROVED BY THE ENFORCEMENT AGENCY. IF THE ANCHOR OR DOVEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER OR RECORD WILL DETERMINE A NEW LOCATION.
16. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH ADHESIVE ANCHORS.
17. ADHESIVE ANCHORS TO BE INSTALLED OVERHEAD ANY DEPTH, OR HORIZONTALLY MORE THAN 8" DEEP, SHALL USE SIMPSON PISTON PLUG DELIVERY SYSTEM. REFER TO ICC-ESR-2538 FOR INSTALLATION GUIDANCE & PISTON PLUG SELECTION.
18. REQUIRED TEST LOADS SHALL BE DETERMINED AS SUMMARIZED IN THE TABLE BELOW:

TENSION LOADS (POUNDS)		
SIMPSON SET-UP (ICC-ES ESR-2506)		
CRACKED CONCRETE, SEISMIC (ASD) CONDITION B		
NOMINAL BAR SIZE	EMBEDMENT DEPTH	NORMAL - WEIGHT CONCRETE
DIA (IN)	H (IN)	F _{cr} = 3000 PSI
#4	2.34	3456
#5	2.6	7540
#6	3	7554
#5	5	1751
#6	9	16965

19. THE TESTING OF THE EXPANSION BOLTS SHALL BE DONE IN THE PRESENCE OF THE PROJECT INSPECTOR AND REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO THE ENFORCEMENT AGENCY.
20. IF ANY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME CATEGORY, NOT PREVIOUSLY TESTED, UNTIL TWENTY (20) CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENTS. THE INITIAL TEST FREQUENCY SHALL BE INCREASED BY 1.33 FOR SEISMIC OR WIND FORCES.
21. THE ACCEPTANCE CRITERIA FOR INSTALLED ANCHORS IS AS FOLLOWS:
- HYDRAULIC RAM METHOD: THE ANCHOR SHOULD HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.

22. IF ANY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME CATEGORY, NOT PREVIOUSLY TESTED, UNTIL TWENTY (20) CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENTS. THE INITIAL TEST FREQUENCY SHALL BE INCREASED BY 1.33 FOR SEISMIC OR WIND FORCES.
23. THE ACCEPTANCE CRITERIA FOR INSTALLED ANCHORS IS AS FOLLOWS:
- HYDRAULIC RAM METHOD: THE ANCHOR SHOULD HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.

TENSION LOADS (POUNDS)		
SIMPSON SET-UP (ICC-ES ESR-2506)		
CRACKED CONCRETE, SEISMIC (ASD) CONDITION B		
NOMINAL BAR SIZE	EMBEDMENT DEPTH	NORMAL - WEIGHT CONCRETE
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#6	9	16965

SIMPSON PRODUCT	LARR #	ICC / ESR / PMO - ER
HTT	25489	ER-0124
HOU	25720	ESR-2330
A36	25716	ESR-2606
C/SYST	25489	ER-0124
M/STAM/ST	25489	ER-0124
CC/CO/CO/CO/CO	25714	ESR-2604
SDS	25711	ESR-2552
FACE MOUNT HANGERS	25701	ESR-2615
TOP PLATE HANGERS	25703	ESR-2615
SET UP (CONCRETE MASONRY)	25729	ESR-1772
SSW	25625	ESR-1670
TITEN HD	25721	ESR-2713

REINFORCING STEEL (FOR CONCRETE AND MASONRY)

1. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 19A OF THE CODE, ASTM A615, GRADE 60 UNO.
2. BARS SHALL BE CLEAN OF RUST, GREASE, OR OTHER MATERIALS LIKELY TO IMPAIR BOND. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185. WWF SHALL BE SUPPORTED ON APPROVED CHAIRS.
4. REINFORCING BAR SPLICES SHALL BE MADE AS INDICATED ON THE DRAWINGS. LAP ALL HORIZONTAL BARS AT CORNERS AND INTERSECTIONS. STAGGER ALL SPLICES UNLESS NOTED OTHERWISE ON PLANS.
5. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL INPLACE INSPECTION IS MADE.
6. WHERE WELDING OF REINFORCING IS APPROVED BY THE STRUCTURAL ENGINEER, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING E70XX OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF STRUCTURAL WELDING CODE-REINFORCING STEEL, AWS-D1.1 LATEST REVISION. REINFORCING BARS TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-706.
7. BARS IN SLABS SHALL BE SECURELY SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS, PRIOR TO PLACING CONCRETE.
8. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "A.C.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" LATEST EDITION.
9. COMPLETE AND DETAILED REINFORCING PLACEMENT DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ARCHITECT FOR APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION IN ACCORDANCE WITH THE SPECIFICATIONS AND APPLICATIONS. THESE DRAWINGS SHALL BE AVAILABLE TO THE JOB SITE PRIOR TO PLACING OF CONCRETE.
10. MILL TEST REPORTS FOR GRADE 60 BARS SHALL BE SUBMITTED PRIOR TO PLACEMENT OF CONCRETE.
11. PROVIDE INSPECTION OF CONCRETE PER SPECIAL INSPECTION NOTES SECTION. INSPECTION SHALL BE SCHEDULED SO THAT PLACEMENT OF REINFORCING STEEL, CONDUIT, SLEEVES, AND EMBEDDED ITEMS MAY BE CORRECTED PRIOR TO PLACEMENT OF OVERLYING GRDS OR REINFORCING STEEL.
12. ALL GRADE 60 REINFORCING STEEL SHALL BE CLEARLY MARKED TO DIFFERENTIATE THEM FROM GRADE 40 REINFORCING STEEL IF CONCURRENTLY ON SITE.
13. CONCRETE PROTECTION FOR REINFORCEMENT:
- (1) CAST-INPLACE CONCRETE (NON-PRESTRESSED), THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:
- | | MIN COVER, (IN) |
|--|-----------------|
| A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH | 3 |
| B. CONCRETE EXPOSED TO EARTH OR WEATHER: NO BAR THRUING, 18 BAR | 2 |
| NO 5 BAR, W31 OR D31 WIRE AND SMALLER | 1-1/2 |
| C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS, JOISTS | 1-1/2 |
| NO 14 BAR AND NO 18 BAR | 3/4 |
| NO 11 BAR AND SMALLER | 3/4 |
| BEAMS AND COLUMNS | 1- |

AREA (IN ²)	
AB	ANCHOR BOLT
ABV	ABOVE
ADDL	ADDITIONAL
ADJ	ADJACENT
AFF	ABOVE FINISH FLOOR
AGGR	AGGREGATE
ALT	ALTERNATE
ALUM	ALUMINUM
ANCH	ANCHOR
APPRVD	APPROVED
APPROX	APPROXIMATELY
ARCH	ARCHITECTURAL; ARCHITECT
AR	ANCHOR ROD
ARND	AROUND
&	AND
@	AT
BEL	BELOW
BD	BOARD
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BM	BEAM
BN	BOUNDARY NAIL
BNDRY	BOUNDARY
BOT OR B	BOTTOM
BOF	BOTTOM OF FOOTING
BRGC	BRACING
BRG	BEARING
BSMT	BASEMENT
BT	BENT
BTW	BETWEEN
BYND	BEYOND
CAM	CAMBER
CALCS	CALCULATIONS
CANT	CANTILEVER
CBC	CALIFORNIA BUILDING CODE
CC	CENTER TO CENTER
CCR	CALIFORNIA CODE OF REGULATIONS
CG	CENTER OF GRAVITY
CIP	CAST IN PLACE
CJ	CONSTRUCTION JOINT; CONTROL JOINT
CL	CENTER LINE
CLR	CLEARANCE; CLEAR
CMU	CONCRETE MASONRY UNIT
CO	COMPANY
COL	COLUMN
COMP	COMPRESSION
CONC	CONCRETE
CONN	CONNECTION; CONNECT
CONST	CONSTRUCTION
CONT	CONTINUE; CONTINUOUS
CONTR	CONTRACTOR
CP	COMPLETE PENETRATION
CTR	CENTER
CTSK	COUNTERSINK; COUNTERSUNK
CF	CUBIC FOOT
d	PENNY (NAIL OR d _b)
DBL	DOUBLE
DBLR	DOUBLER
DEPT	DEPARTMENT
DET	DETAIL
DF	DOUGLAS FIR
DIA OR DIA	DIAMETER
DIAG	DIAGONAL
DIAPH	DIAPHRAGM
DIM	DIMENSION
DIR	DIRECTION
DIST	DISTANCE
DL	DEAD LOAD
DN	DOWN
DO	DITTO
DWG	DRAWING
DWL	DOWEL
(E)	EXISTING
EA	EACH
EF	EACH FACE
e.g.	FOR EXAMPLE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
EMB	EMBEDDED
EN	EDGE NAIL
ENGR	ENGINEER
EOS	EDGE OF SLAB
EQ	EQUAL
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
ES	EACH SIDE
ETC	ET CETERA
EW	EACH WAY
EXIST	EXISTING
EXT	EXTERIOR
Fb	ALLOWABLE BENDING STRESS
FB	FLAT BAR
F _c	CONCRETE COMPRESSION STRENGTH
FD	FLOOR DRAIN
FDN	FOUNDATION

HD	HOLD DOWN
HDR	HEADER
HGR	HANGER
HK	HOOK
HORIZ OR H	HORIZONTAL
HOSP	HOSPITAL
HP	HEAVY PILING
HR	HARD ROCK
HS	HIGH STRENGTH
HT	HEIGHT
I	MOMENT OF INERTIA
ID	INSIDE DIAMETER
i.e.	THAT IS
IF	INSIDE FACE
IN	INCH
INCL	INCLUDE, INCLUDING
INFO	INFORMATION
INSP	INSPECTION, INSPECTOR
INT	INTERIOR
INTERM	INTERMEDIATE
INV	INVERT
JST	JOIST
JT	JOINT
KIP OR K	1,000 POUNDS
KO	KNOCK-OUT
KP	KING POST
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
LAB	LABORATORY
LBS OR #	POUND
LDGR	LEDGER
LF	LINEAL FOOT
LG	LONG
LH	LOW HYDROGEN
LIN	LINEAL; LINEAR
LL	LIVE LOAD
LLBH	LONG LEGS BACK-TO-BACK
LLB	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LOCS	LOCATIONS
LONGIT	LONGITUDINAL
LP	LOW POINT
LSH	LONG SLOTTED HOLES
LTWT	LIGHTWEIGHT
LVL	LEVEL
MAS	MASONRY
MAT	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MDF	MEDIUM DENSITY FIBERBOARD
MECH	MECHANICAL
MEZZ	MEZZANINE
MFR	MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
MK	MARK
MTL	METAL
MULT	MULTIPLE
(N)	NEW
N	NORTH
NAT	NATURAL
NF	NEAR FACE
NIC	NOT IN CONTRACT
NLG	NAILING
NO. OR #	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
OPNG	OPENING
OSB	ORIENTED STRAND BOARD
PARA OR //	PARALLEL
PC	PRECAST
PC	PIPE COLUMN
PCF	POUNDS PER CUBIC FOOT
PCI	POUNDS PER CUBIC INCH
PDF	POWER DRIVEN FASTENERS
PLF	POUNDS PER LINEAL FOOT
PERP OR ⊥	PERPENDICULAR
PI	PLYWOOD INDEX
PL OR PL	PLATE
PL	PROPERTY LINE
PLY	PLYWOOD
PLCS	PLACES
PLBG	PLUMBING
PNL	PANEL
PP	PARTIAL PENETRATION
PREFAB	PREFABRICATED
PRKG	PARKING
PROJ	PROJECT
PROP	PROPERTY
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PUN	PUNCHED

SHT	SHEET
SHTG	SEATHING
SIM	SIMILAR
SJ	SEISMIC JOINT
SLBB	SHORT LEGS BACK-TO-BACK
SMS	SHEET METAL SCREWS
SPA	SPACES
SPCG	SPACING
SPCL	SPECIAL
SPCS	SPACES
SPECS	SPECIFICATIONS
SQ	SQUARE
SS	SELECT STRUCTURAL
SSH	SHORT SLOTTED HOLES
STAGG	STAGGER
STD	STANDARD
STIFF	STIFFENER
STIRR	STIRRUP
STL	STEEL
STRUC	STRUCTURAL
SW	SHEAR WALL
SYM	SYMMETRICAL

T	TOP
T&B	TOP & BOTTOM
T&G	TONGUE & GROOVE
T/	TOP OF
TB	TIE BEAM
TEMP	TEMPERATURE, TEMPORARY
THK	THICK
THRU	THROUGH
TL	TOTAL LOAD
TN	TOE NAIL
TOB	TOP OF BEAM
TOC	TOP OF CURB
TOL	TOP OF LEDGER
TOS	TOP OF STEEL
TOSH	TOP OF SHEAR WALL
TOSL	TOP OF SLAB
TOW	TOP OF WALL
TRANSV	TRANSVERSE
TSR	TAPERED STEEL GIRDER
TYP	TYPICAL

UL	UNDERWRITERS' LABORATORY, INC.
UBC	UNIFORM BUILDING CODE
UNO	UNLESS NOTED OTHERWISE
UT	ULTRASONIC TEST

VERT or V	VERTICAL
VIF	VERIFY IN FIELD

WF	WIDE FLANGE
WI	WITH
W/C	WATERCEMENT
W/O	WITHOUT
WD	WOOD
WP	WORK POINT, WATERPROOF
WWF	WELDED WIRE FABRIC
WHT	WEIGHT

SYMBOLS

DETAIL NUMBER	
SHEET NUMBER	
DETAIL REFERENCE	
SECTION REFERENCE	
WALL ELEVATION	
DATUM ELEVATION	
REVISION & CLOUD	
GRID SYSTEM	

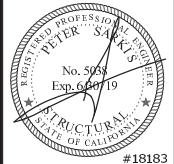
3. ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF CBC CH 17A AND ANY ADDITIONAL REQUIREMENTS SET FORTH IN THESE DRAWINGS AND/OR THE PROJECT SPECIFICATIONS.
4. THE OWNER SHALL PROVIDE FOR A SPECIAL INSPECTOR(S) WHO WILL PROVIDE SPECIAL INSPECTIONS OF THE CONSTRUCTION AS PRESCRIBED BY THE CODE AND THE CONSTRUCTION DOCUMENTS, IN NO CASE SHALL THE SPECIAL INSPECTOR BE REQUIRED TO PROVIDE MORE THAN FIVE HOURS LESS THAN THE MINIMUM REQUIREMENTS OF THE CODE OR ANY OTHER GOVERNING AUTHORITY. ANY SUCH DISCREPANCY SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ENGINEER.
5. THE SPECIAL INSPECTOR SHALL BE QUALIFIED BY THE JURISDICTION TO PROVIDE INSPECTIONS OF THE SPECIFIC CONSTRUCTION OPERATION REQUIRED, PROVIDED THAT THE SPECIAL INSPECTOR SHALL BE REPORTED TO THE ENGINEER FIRST BY EACH OF HIS INSPECTIONS.
6. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE ENGINEER, ARCHITECT AND THE CONTRACTOR IN A TIMELY MANNER. ALL DISCREPANCIES BETWEEN THE INSPECTORS WORK AND THE CONTRACT DOCUMENTS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. THEN, IF NOT CORRECTED TO THE ENGINEER, ARCHITECT AND THE BUILDING OFFICIAL, THE SPECIAL INSPECTOR SHALL BE REQUIRED TO CORRECT THE DISCREPANCY. ALL PARTIES CAN IDENTIFY THE OUTSTANDING WORK THAT NEEDS CORRECTION AND SUCH THAT CORRECTED WORK IS RECORDED IN A TIMELY MANNER.
7. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE SPECIAL INSPECTOR'S KNOWLEDGE, CONFORMED TO THE CODE, THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE. IF NONCONFORMING WORK REMAINS, THEN THIS WORK SHALL BE IDENTIFIED AND NOTED IN THE REPORT.
8. THE CONTRACTOR SHALL PROVIDE ACCESS TO THE WORK REQUIRING SPECIAL INSPECTION TO THE SPECIAL INSPECTOR. THE SPECIAL INSPECTOR SHALL, ON SITE, FOR THE SPECIAL INSPECTOR TO RECEIVE CORRESPONDENCE AND AN AREA TO HAVE CORRESPONDENCE FOR THE BUILDING OFFICIAL AND THE CONTRACTOR. THE SPECIAL INSPECTOR SHALL MAINTAIN A QUALITY CONTROL INDIVIDUAL THAT SHALL ACT AS THE MAIN POINT OF CONTACT FOR THE INSPECTOR, ENGINEER AND ARCHITECT REGARDING INSPECTIONS ISSUES.
9. A PRE-INSPECTION MEETING SHALL BE HELD AND CONVENED BY THE CONTRACTOR ONCE INSPECTION HAS STARTED. AS A MINIMUM, THE ENGINEER, THE SPECIAL INSPECTOR, THE ARCHITECT, THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR'S QUALITY CONTROL INDIVIDUAL SHALL BE INVITED TO ATTEND. THE PURPOSE OF THE MEETING WILL BE TO MEET AND DISCUSS THE PROJECT'S QUALITY CONTROL PLAN, ESTABLISH COMMUNICATION PROTOCOLS AND ANSWER ANY QUESTIONS.

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCE STANDARD	CBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	X	ACI 318 Ch. 20, 25.2, 25.3, 26.5.1-26.5.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16" AND c. INSPECT ALL OTHER WELDS.	- X	X 	AWS D14 ACI 318: 26.5.4	-
3. INSPECT ANCHORS CAST IN CONCRETE	-	X	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS: a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS; b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	X 	 X	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C172 ASTM C31 ACI 318: 26.4.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	ACI 318: 26.4.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318: 26.4.7-26.4.9	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES; AND b. GROUTING OF BONDED PRESTRESSING TENDONS	X X	- 	ACI 318: 26.9.2.1 ACI 318: 26.9.2.3	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	-	X	ACI 318: Ch. 26.8	-
11. VERIFY IN-SITU CONCRETE STRENGTH. PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X	ACI 318: 26.10.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	ACI 318: 26.10.7-1.(b)	-

a. WHERE APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE

[illegible]

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TESTING, INSPECTION & ABBREVIATIONS

STILES RESIDENCE REMODEL

87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

REVIEWED BY / DATE

FIRST SUBMITTAL DATE
10/11/2018

DRAWN BY

JOB NUMBER
2018-183

SHEET NUMBER

S-0.2

LEGEND:

1.

INDICATES TOP OF SLAB ELEVATION FROM 0'-0"
2.

INDICATES TOP OF FOOTING ELEVATION.
3.

INDICATES SLAB DEPRESSION
4.

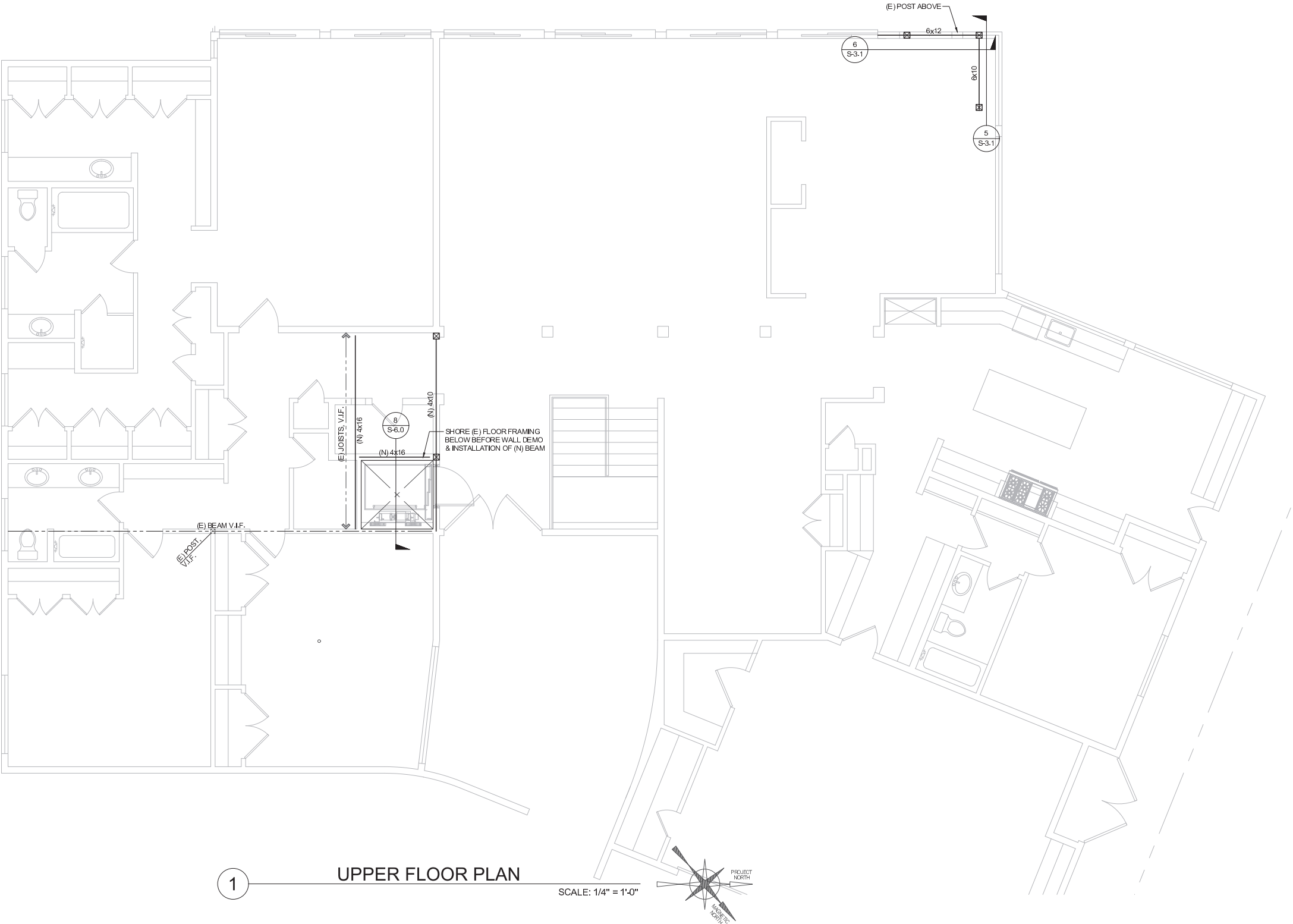
INDICATES WOOD POST.
5.

INDICATES SIMPSON STRONG WALL
6.

INDICATES (N) CONC. WALL

FRAMING NOTES:

1. FOR GENERAL NOTES AND ABBREVIATIONS SEE S-0.1 AND S-0.2
2. FOR TYPICAL CONCRETE DETAILS SEE S-3.0 & S-3.1
3. FOR TYPICAL WOOD DETAILS SEE S-6.0
4. REFER TO ARCHITECTURAL DRAWINGS FOR CURBS, RAMPS, DRAINS, EXTERIOR SLABS, PITS, SWALES, TRENCHES, OPENINGS, DIMENSIONS, TOP OF SHEATHING & TOP OF PARAPET ELEVATIONS NOT SHOWN ON PLANS, ETC.
5. VERIFY ALL DIMENSIONS, ELEVATIONS, FINISH SURFACES, SLOPES, DRAINS DEPRESSIONS, CURBS ETC. WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION.
6. SEE CIVIL & LANDSCAPE DRAWINGS FOR ELEVATIONS AND SLOPES.
7. CONTRACTOR TO VERIFY & COORDINATE LOCATIONS OF MECHANICAL UNITS WITH MECHANICAL ENGINEER.
8. REFER TO ARCH PLANS FOR DEMO PLAN SHOWING EXISTING WALLS TO REMAIN. WALL SHEATHING AT EXISTING WALLS TO BE INSTALLED ON OPPOSITE SIDE OF STUCCO.
9. ALL NEW FOOTINGS TO BEAR DIRECTLY ON OR IN BEDROCK. BEDROCK ELEVATION TO BE VERIFIED BY GEOTECHNICAL ENGINEER.
10. NOT USED
11. DRILL & EPOXY ALL (N) VERT. & HORIZ. REINF. INTO (E) CONC. WALL & FTG WHERE INTERRUPTED. MIN. 6" EMBED, TYP

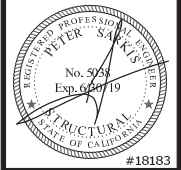


UPPER FLOOR PLAN

SCALE: 1/4" = 1'-0"

REVISIONS		
NO.	DATE	BY

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UPPER FLOOR PLAN

STILES RESIDENCE REMODEL

87 MONARCH BAY DRIVE
DANA POINT, CALIFORNIA 92629

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DRAWN BY	JOB NUMBER 2018-183
SHEET NUMBER S-2.1	

NOT USED

SCALE: NTS

10

MANDATORY MINIMUM FORMWORK

SCALE: NTS

7

NOT USED

SCALE: NTS

11

NOT USED

SCALE: NTS

8

		CLASS 'B' TENSION LAP SPLICE LENGTH									
		#3	#4	#5	#6	#7	#8	#9	#10	#11	
CONCRETE CLASS 'B' TENSION SPLICE	f _c (psi)	BAR SIZE GR 60									
		BAR DIAMETER, d _b (in)	0.375	0.500	0.625	0.750	0.875	1.000	1.128	1.270	1.410
	3000	TOP BAR	2'-4"	3'-1"	3'-11"	4'-8"	6'-9"	7'-9"	8'-9"	9'-10"	10'-11"
		BOTTOM BAR	1'-10"	2'-5"	3'-0"	3'-7"	5'-3"	6'-0"	6'-9"	7'-7"	8'-5"
4000/4500	TOP BAR	2'-0"	2'-8"	3'-4"	4'-1"	5'-11"	6'-9"	7'-7"	8'-6"	9'-5"	
	BOTTOM BAR	1'-7"	2'-1"	2'-7"	3'-1"	4'-6"	5'-2"	5'-10"	6'-7"	7'-3"	
5000	TOP BAR	1'-10"	2'-5"	3'-0"	3'-8"	5'-3"	6'-0"	6'-9"	7'-7"	8'-6"	
	BOTTOM BAR	1'-5"	1'-10"	2'-4"	2'-10"	4'-1"	4'-8"	5'-3"	5'-10"	6'-6"	

NOTES:

1. ALL REINFORCING MUST MEET ONE OF THE FOLLOWING CONDITIONS.

A. CASE I: THE CLEAR SPACING OF THE BARS BEING DEVELOPED OR SPLICED IS NOT LESS THAN ONE BAR DIAMETER (d_b)

CLEAR COVER NOT LESS THAN ONE BAR DIAMETER (d_b) AND STIRRUPS OR TIES THROUGHOUT THE SPLICE LENGTH.

B. OR CASE II: THE CLEAR SPACING OF THE BARS BEING SPLICED IS NOT LESS THAN TWO BAR DIAMETERS (2d_b) AND THE CLEAR COVER IS NOT LESS THAN ONE BAR DIAMETER (d_b)

C. FOR ALL OTHER CASES MULTIPLY THE SPLICES SHOWN BY 1.5.

2. THE ABOVE VALUES ARE FOR NORMAL WEIGHT CONCRETE.

3. THE ABOVE VALUES ARE FOR UNCOATED REINFORCEMENT.

4. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF NEW CONCRETE PLACED BELOW THE BAR.

5. BOTTOM BARS ARE ALL VERTICAL BARS AND HORIZONTAL REINFORCEMENT WITH LESS THAN 12" OF NEW CONCRETE PLACED BELOW THE BAR.

MINIMUM BEND DIAMETER (STIRRUPS AND TIES)				
BAR SIZE	#3	#4	#5	#6
"D"	1.1/2"	2"	2.1/2"	4.1/2"
"a"	3"	3"	3.3/4"	9"
"b"	3"	3"	3.3/4"	4.1/2"

STANDARD HOOK DETAIL FOR STIRRUPS AND TIES

MINIMUM BEND DIAMETER (PRINCIPAL REINFORCEMENT)			
BAR SIZE	#3 THRU #8	#9 THRU #11	#14 & #18
"D"	6db	8db	10db

STANDARD HOOK DETAIL FOR PRINCIPAL REINFORCEMENT

WHERE: "a" IS THE CLEAR COVER

"b" IS THE CENTER TO CENTER SPACING

"db" IS THE BAR DIAMETER

SPLICE LENGTHS FOR BARS PLACED SHALL MEET THE FOLLOWING (SEE NOTE 1):	
CASE I	CASE II
a<db: ANY b	a<db: b>3db
a<db: b<3db	-

TYPICAL NWC (CLASS B) LAP SPLICE SCHEDULE

SCALE: NTS

2

NOT USED

SCALE: NTS

12

NOT USED

SCALE: NTS

9

NOT USED

SCALE: NTS

3

REVISIONS		
NO.	DATE	BY

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#18183

TYPICAL CONCRETE DETAILS

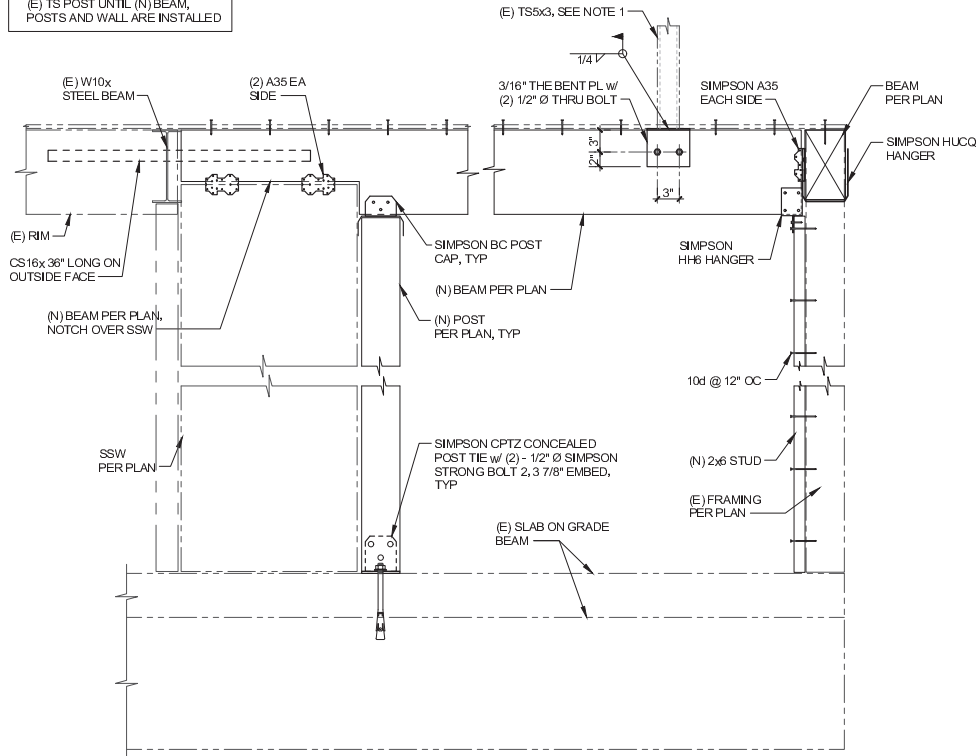
STILES RESIDENCE REMODEL

87 MONARCH BAY DRIVE

DANA POINT, CALIFORNIA 92629

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FIRST SUBMITTAL DATE	
10/11/2018	
DRAWN BY	JOB NUMBER
2018-183	
SHEET NUMBER	
S-3.0	

NOTE:
1. CONTRACTOR TO PROVIDE
TEMPORARY SHORING FOR
(E) TS POST UNTIL (N) BEAM,
POSTS AND WALL ARE INSTALLED



DETAIL

SCALE: NTS

6

NOT USED

SCALE: NTS

11

NOT USED

SCALE: NTS

7

NOT USED

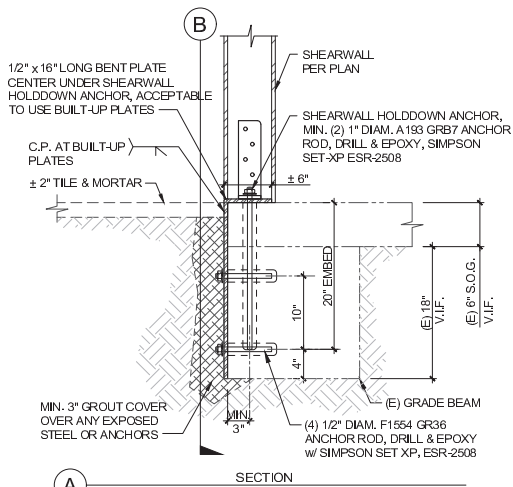
SCALE: NTS

12

NOT USED

SCALE: NTS

8

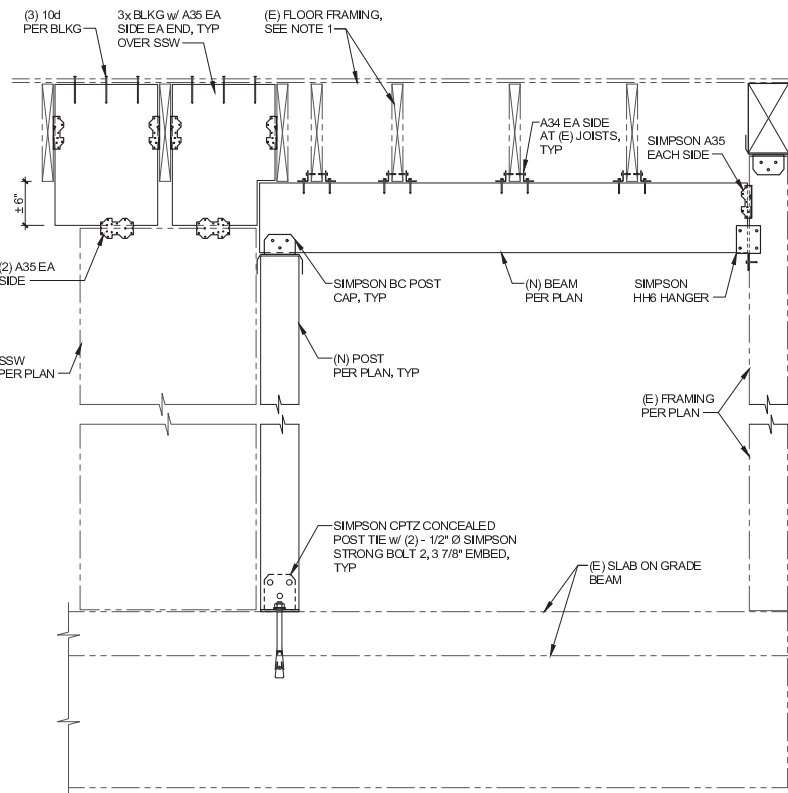


SECTION AT FOUNDATION

SCALE: NTS

4

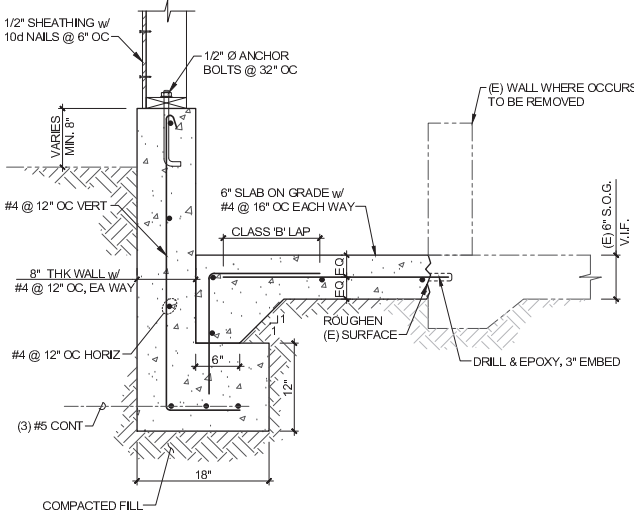
NOTE:
1. CONTRACTOR TO PROVIDE TEMPORARY SHORING FOR (E)
FRAMING UNTIL (N) BEAM, POSTS AND WALL ARE INSTALLED.



DETAIL

SCALE: NTS

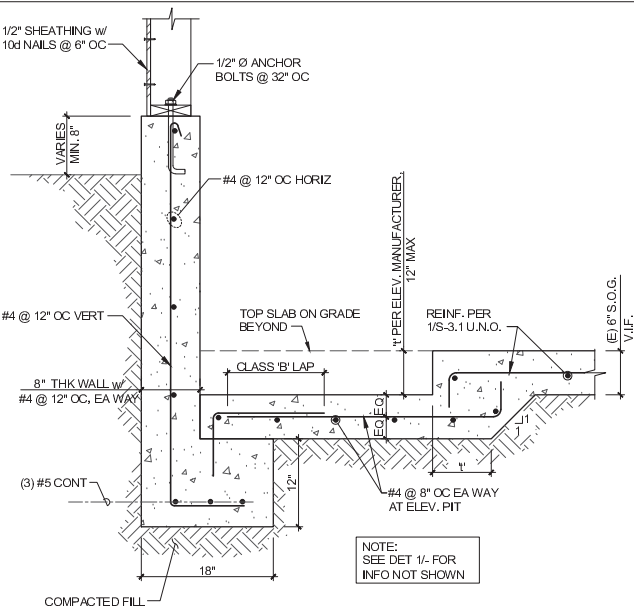
5



SECTION

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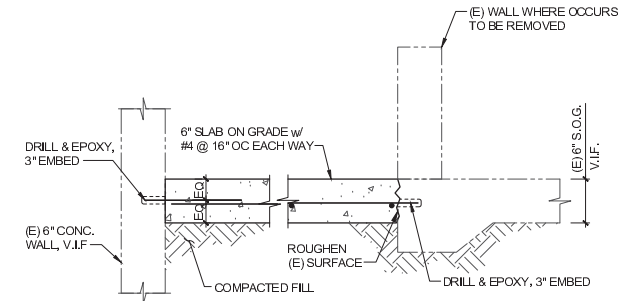
1



SECTION

SCALE: NTS

2



SECTION

SCALE: NTS

3

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CONCRETE DETAILS

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SHEET NUMBER S-3.1	

NOT USED

SCALE: NTS

10

TYPICAL FLOOR / ROOF OPENING DETAIL

SCALE: NTS

7

NON-BEARING WALL FRAMING

SCALE: NTS

1

NOT USED

SCALE: NTS

11

SECTION AT ELEVATOR OPENING

SCALE: NTS

8

TYPICAL HEADER CONNECTION DETAIL

SCALE: NTS

2

TYPICAL BACKING

SCALE: NTS

2

NOT USED

SCALE: NTS

12

NOT USED

SCALE: NTS

9

TYPICAL TOP SPLICE CONNECTION

SCALE: NTS

6

TYP FLUSH BEAM TO PERPENDICULAR WALL

SCALE: NTS

3

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#18183

TYPICAL WOOD DETAILS

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SHEET NUMBER

S-6.0